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# Trends and progress in AnMBR for domestic wastewater treatment and their impacts on process efficiency and membrane fouling

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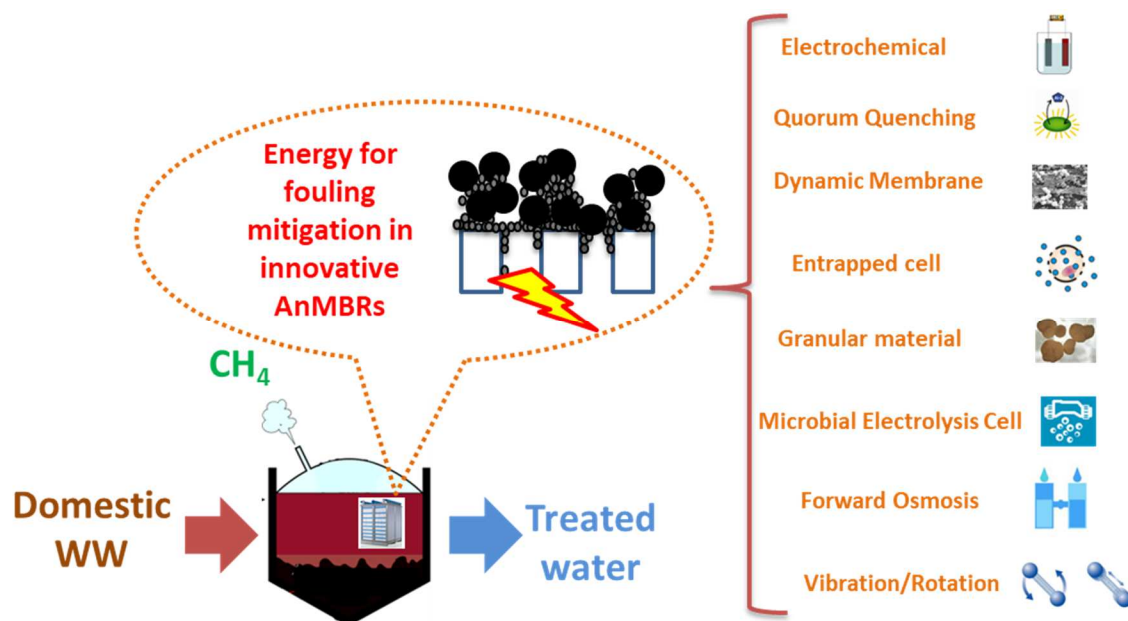
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## Keywords

Anaerobic Membrane Bioreactor, Domestic wastewater treatment, Fouling, Hybrid processes, Biogas.

## Graphical abstract



## Abstract

Anaerobic membrane bioreactor has emerged as an innovative technology in treating domestic wastewater due to its excellent produced effluent quality and high potential of neutral or positive energy balance. One of the biggest challenges in positive energy objective is fouling mitigation which contributes towards 70% of the total energy requirement of MBR-

22 based domestic wastewater treatment. Numerous studies were carried out to address this  
 23 issue, utilizing various reactor design configurations and operating conditions for energy  
 24 minimization as well as membrane performance enhancement. The latest research trend in  
 25 this sector is the establishment of hybrid processes like Granular Anaerobic Membrane  
 26 Bioreactors (G-AnMBR), Forward Osmosis Anaerobic Membrane Bioreactor (FO-AnMBR)  
 27 and Microbial Electrolysis Cell - Anaerobic Membrane Bioreactor (MEC-AnMBR) for  
 28 domestic wastewater treatment which not only provides efficiency in treatment but also  
 29 improves fouling mitigation. Also, the application of techniques developed particularly for  
 30 fouling mitigation like quorum quenching and sensing, cell entrapment and membrane  
 31 module vibrations in AnMBRs were assessed. This paper reviews the latest trends in  
 32 anaerobic membrane bioreactors research with regards to water quality produced, removal  
 33 efficiencies and fouling mitigation.

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## 75 **Table of Abbreviations**

76	AeMBR: Aerobic membrane bioreactor
77	AHL: N-acryl homoserine lactone
78	AnDMBR: Anaerobic dynamic membrane bioreactor
79	AnEMR: Anaerobic electrochemical membrane bioreactor
80	AFMBR: Anaerobic fluidized membrane bioreactor
81	AnGS: Anaerobic granular sludge
82	AnMBR: Anaerobic membrane bioreactor
83	ATP: Adenosine triphosphate
84	BB: Building blocks
85	GSP: Biogas sparging
86	CFV: Crossflow velocity
87	COD: Chemical oxygen demand
88	CSTR: Continuous stirred tank reactor
89	DLVO: Derjaguin, Landau, Verwey, and Overbeek
90	DM: Dynamic membrane
91	DWW: Domestic wastewater
92	DWWT: Domestic wastewater treatment

93 EB: Empty beads  
94 EG-AnMBR: Externally submerged granular AnMBR  
95 IG-AnMBR: Internally submerged granular AnMBR  
96 ES-AnMBR: Externally submerged membrane bioreactor  
97 EPS: Extracellular polymeric substances  
98 F/M ratio: Food to microorganism ratio  
99 FO-AnMBR: Forward Osmosis Anaerobic Membrane Bioreactor  
100 GAC: Granular activated carbon  
101 G-AnMBR: Granular anaerobic membrane bioreactor  
102 GHG: Greenhouse gases  
103 GSI<sub>m</sub>: Biogas sparging intensity  
104 GSP: Biogas sparging  
105 HRT: Hydraulic residence time  
106 LMH: Liter per square meter per hour  
107 LMW: Low molecular weight  
108 MF: Microfiltration  
109 MEC: Microbial electrolysis cell  
110 MEC-AnMBR: Microbial Electrolysis Cell - Anaerobic Membrane Bioreactor  
111 MLSS: Mixed liquor suspended solids  
112 MMV: Magnetically induced membrane vibration  
113 MT: Membrane tank  
114 MWh: Megawatt hour  
115 P/C ratio: Protein to carbohydrate ratio  
116 PSP: Particle sparging  
117 PVDF: Polyvinylidene difluoride  
118 QQ: Quorum quenching  
119 QQB: Quorum quenching beads  
120 RMEM: Rotating membrane  
121 SCSA: Specific cathode surface area  
122 SDG: Sustainable development goal  
123 SG-AnMBR: Internally submerged granular AnMBR  
124 SMP: Soluble microbial products  
125 SRT: Solid retention time  
126 TMP: Trans-membrane pressure  
127 UASB: Up-flow anaerobic sludge blanket  
128 UF: Ultrafiltration  
129 VLR: Volumetric Loading Rate  
130 VSEP: Vibratory shear enhancement process  
131 WWTP: Wastewater Treatment Plants  
132 WRRF: Water Resource Recovery Facilities  
133

## 134 **1. Introduction**

135 In 2015, United Nations adopted the 2030 Agenda for Sustainable Development (SD) (UN  
136 2019); SD Goal (SDG) 6 aims at ensuring availability and sustainable management of water  
137 and sanitation for all. To help address these issues, domestic wastewater (DWW) is thus now  
138 considered to be an alternative water (SDG 6), nutrients (SDG 2) and energy (SDG 7) source  
139 for various applications. Wastewater Treatment Plants (WWTPs) have even been rebranded  
140 as Water Resource Recovery Facilities (WRRFs) to enlighten the significant resource  
141 recovery potential that exists in DWW streams (Jain, 2018; Fulcher, 2014). The biological  
142 treatments such as aerobic and partial nitrification-anammox granular sludge are becoming  
143 increasingly popular in DWW treatment due to their high tolerance of the toxicity of feed,  
144 compactness, settling efficiencies and effectiveness in treating complex effluents with high  
145 organic load and low biodegradable compounds (Baeten et al., 2019; Taghipour, 2017;  
146 Taghipour and Ayati, 2017; Zhang et al., 2016). However, aerobic processes are highly  
147 energy-intensive and produce a large amount of sludge (Kim et al., 2011).

148 Recently, a change of paradigm was made from reliance on aerobic treatment towards  
149 anaerobic treatment which uses organic carbon from the DWW to produce energy in the form  
150 of methane gas and reduces the amount of sludge to be handled. However, anaerobic  
151 treatment alone may not be enough to meet stringent discharge limits and to hold anaerobic  
152 micro-organisms that have a very slow-growth rate. The introduction of membrane  
153 technology into anaerobic bioreactor is a promising solution to these issues.

154 Since more than twenty years, Anaerobic Membrane Bioreactors (AnMBRs) are in operation  
155 for the treatment of high strength WW i.e. industrial and agricultural WW (Amha et al., 2019;  
156 Bu et al., 2017; Thongmak et al., 2016). Kanai et al., (2010) have presented the successful  
157 demonstration of Kubota's submerged anaerobic membrane biological reactor (a patented  
158 technology with 15 full-scale plants) at a spillage treatment plant in Japan. In addition to that

159 its' applications include food waste and garbage treatment. The technology combines  
160 submerged membrane separation and anaerobic digestion in a single unit with a COD  
161 removal efficiency of up to 92% and an energy recovery of around 12 GJ/d. Christian et al.,  
162 (2011) have reported the two years operation of a full-scale AnMBR treating food industry  
163 wastewater achieving 99% COD removal and resulting in up to 50% reduction in the total  
164 operating expenses of the wastewater treatment plant facility. For domestic wastewater  
165 treatment, no full-scale plant has been commissioned so far owing to the highly variable feed  
166 conditions and fouling issues. Robles et al., (2018) have underlined the need of further works  
167 on fouling behavior understanding, fouling mitigation and process control in AnMBRs for  
168 DWW treatment. New trends in circular economy calls for the utilization of AnMBR for  
169 domestic wastewater treatment since AnMBRs are able to:

- 170 • Retain anaerobic bacteria completely (Hydraulic and Solid Retention Time (HRT and  
171 SRT) are uncoupled in AnMBR) and work with higher loading capacity;
- 172 • Produce excellent permeate qualities (i.e. high removal of suspended solids, organic  
173 matter and microorganisms) thanks to microfiltration (MF) or ultrafiltration (UF)  
174 membranes (Lim et al., 2020);
- 175 • Keep the nutrients of the influent available for their recovery or direct reuse (e.g. struvite  
176 crystallization, microalgae cultivation, fertigation...) (Judd et al., 2015);
- 177 • Reduce Green House Gas (GHG) emissions by saving energy consumption: net energy  
178 production of 0.1 kWh/m<sup>3</sup> with AnMBR when aerobic treatment consumes approximately  
179 0.25-0.6 kWh/m<sup>3</sup> of treated DWW (Pretel et al., 2016; McCarty et al., 2011).
- 180 • Reduce the amount of sludge to dispose of due to the lower growth yield of the anaerobic  
181 biomass (up to 90% reported by a study (Jeison et al., 2008));
- 182 • Reduce footprint due to higher compacity of AnMBR (15 kg<sub>COD</sub>/m<sup>3</sup>/d) compared to  
183 conventional activated sludge (CAS) systems (4 kg<sub>COD</sub>/m<sup>3</sup>/d).

184 Recent studies have shown the potential applicability of AnMBR for mainstream DWWT  
185 (Maaz et al., 2019; Galib et al., 2016; Pretel et al., 2016). Extensive work on AnMBR has  
186 been published at pilot scale to pave the path for its full-scale implementation (Shin and Bae,  
187 2018). Nevertheless, membrane fouling remains one of the most challenging issues impeding  
188 the development of AnMBRs (Maaz et al., 2019; Saleem et al., 2016), especially with high  
189 free biomass concentration widely used in conventional AnMBRs.

190 Membrane fouling is an inevitable phenomenon and conventional AnMBRs mostly make use  
191 of biogas sparging (GSP) or crossflow velocity (CFV) to remove (or limit) fouling by  
192 promoting turbulences close to the membrane surface (Hu et al., 2007). Total energy  
193 requirements of AnMBR with biogas sparging were reported to be from 0.038 to 5.68  
194 kWh/m<sup>3</sup> (Martin et al., 2011) and crossflow velocity was reported to consume from 3 to 7.3  
195 kWh/m<sup>3</sup> (Aslam et al., 2017) depending upon CFV and mixed liquor suspended solids  
196 (MLSS) of the reactor. It has been reported that fouling mitigation could account for up to  
197 70% of total energy consumption in AnMBRs (Shin and Bae, 2018). To design an energy-  
198 efficient fouling mitigation technique, many studies were carried out during the last decade  
199 understanding fouling behavior (Maaz et al., 2019; Charfi et al., 2017b; Chen et al., 2017a;  
200 Xiong et al., 2016; Ding et al., 2015; Chen et al., 2014; Charfi et al., 2012; Herrera-Robledo  
201 et al., 2011) and investigating various fouling control strategies (Robles et al., 2018; Shin et  
202 al., 2018; Krzeminski et al., 2017; Aslam et al., 2014). Fouling mechanisms in AeMBR  
203 (Aerobic Membrane Bioreactor) and AnMBR were compared and were found to be quite  
204 different. In AeMBR, adenosine triphosphate (ATP) concentration was 30-fold higher than in  
205 AnMBR which leads to higher microbial activity and results in a large fraction of  
206 extracellular polymeric substances (EPS) on the membrane surface. Thus, the main  
207 contributors to fouling in AeMBR arise from microbial activity during substrate  
208 biodegradation. In AnMBR, soluble microbial products (SMP) levels are up to 5 times higher



209 and with higher molecular weights than in AeMBR (i.e; 2526 kDa and 180.1 kDa of  
210 carbohydrates detected in the SMP and 630-640 kDa and 0.9 kDa of protein MW detected in  
211 SMP for AnMBR and AeMBR respectively). This caused a higher rejection of proteins and  
212 carbohydrates in AnMBR which further deposit on the membrane surface thereby resulting in  
213 more severe organic fouling and biofouling in AnMBRs (Xiong et al., 2016). It has also been  
214 reported that inorganic fouling propensity is higher in AnMBR than AeMBR due to higher  
215 carbonate, bicarbonate, ammonia and phosphate concentrations in anaerobic reactors (Lin et  
216 al., 2013).

217 It has been established that the fouling in AnMBRs has remained one of the biggest hurdles  
218 in positive energy treatment of DWWT (Maaz et al., 2019). Thus, the focus of this review is,  
219 particularly on fouling. It aims to present the work that has been done so far in understanding  
220 and optimizing fouling mitigation in AnMBRs for domestic wastewater treatment, highlights  
221 the different potential technologies in this regard and provides researchers, working in this  
222 area, with information that can help in giving them directions for future research work.

223 The paper has been divided in to two major sections, the first will shed light on the current  
224 state of the art based on the return of experience and progresses of AnMBR at the pilot-scale  
225 for DWWT. The aim of this section is to inform the readers with the basics of fouling  
226 mechanism in the AnMBRs, the important definitions and concepts for the understanding of  
227 fouling behaviors, factors effecting and fouling control. It will also provide an overview of  
228 the latest research trends for fouling mitigation strategies in such systems. In the second  
229 section, emerging novel configurations like granular AnMBR (G-AnMBR), AnMBR  
230 combined with forward osmosis membrane (FO-AnMBR), entrapped cell-based AnMBR,  
231 dynamic AnMBR (AnDMBR) and microbial electrolysis cell AnMBR (MEC-AnMBR) will  
232 be discussed in term of removal efficiencies and fouling. These configurations are selected  
233 based on their contribution to fouling control. They were designed by researchers to provide

234 solution to the issues of energy intensive fouling control and enhance membrane operation  
235 with both technical and economic optimization. Respecting the review's focus on fouling,  
236 other innovative techniques that are particularly developed to tackle fouling like quorum  
237 quenching and rotation/vibration application are also discussed. Some insight on methane  
238 production and potential energy generation are also provided.

## 239 **2. Current status of operation and fouling in pilot-scale**

### 240 **AnMBR**

241 The majority of pilot-scale AnMBR systems were designed using commercially available  
242 membrane modules and were fed with real or synthetic domestic wastewaters to reflect  
243 seasonal and daily variations of the wastewater composition. Data acquired from various  
244 pilot-scale AnMBR experiments is crucial to upscale, design and develop full-scale AnMBR.  
245 This section reviews the latest outcomes of the pilot-scale AnMBR systems for DWW  
246 treatment to extract practical data for the commercial use of AnMBR based plants in the  
247 future.

#### 248 **2.1. Configuration models of pilot-scale AnMBRs for fouling control**

249 This section deals with the evaluation of 13 pilot-scale AnMBR studies performed from 2011  
250 to 2019 (Table 1). The two-stage reactor was used by 12 pilot-scale AnMBRs, featuring  
251 anaerobic reactor with an externally submerged membrane tank (MT). Only one AnMBR  
252 system with gas sparging (Config-9) having one stage integrated with an up-flow anaerobic  
253 sludge blanket (UASB) combined with a submerged type membrane in the upper portion of  
254 the UASB reactor system was reported. In the case of a 2-stage reactor configuration, the  
255 membrane is immersed in a separate tank so that intensive shear stress can be applied only on  
256 the membrane to control fouling. Moreover, in situ membrane chemical cleaning can be  
257 performed in the membrane tank without endangering the whole active biomass. The

258 retentate from the membrane tank is circulated back into the anaerobic reactor to enhance the  
259 degradation of organic compounds. In config-12, instead of the continuous stirred tank  
260 reactor (CSTR) and separate membrane tank, anaerobic fluidized bed reactor (AFBR) and  
261 anaerobic fluidized membrane bioreactor (AFMBR) are used in 2-stages (Config-12; Table-  
262 1).

263 The bibliographic study of the 13 AnMBRs (Table-1) shows that three different scouring  
264 approaches were used: gas sparging (GSP) (Config-1 to Config-11), particle sparging (PSP)  
265 (Config-12) and rotating membrane (RMEM) (Config-13). Most of the membranes were  
266 hollow-fiber (HF) submerged membrane except the first one which was a flat sheet (FS)  
267 membrane module called GSP-FS (Config-1). AnMBR systems used ultrafiltration (UF)  
268 membranes except for GSP-HF<sub>MF</sub> (Config-6) which featured a MF membrane.

269 Apart from gas sparging, particle sparging and membrane rotation, some additional  
270 mechanisms were also employed in different configurations. In Config-1, circulating sludge  
271 was used to create additional shear to control membrane fouling. Ventilation and  
272 backwashing mechanisms were used in Config-3 and Config-4. Backwashing was used as an  
273 anti-fouling mechanism in Config-6 and Config-9. Chemical cleaning was employed for  
274 Config-7 and Config-8. Section 2.3 contains a detailed discussion regarding those different  
275 anti-fouling strategies.

## 276 **2.2. Fouling mechanisms in pilot-scale AnMBRs**

277 Membrane fouling is indicated by many factors like increased transmembrane pressure  
278 (TMP) (at constant flux), reduced flux (at constant TMP), presence of extracellular polymeric  
279 substances (EPS) and soluble microbial products (SMP) in the bulk phase. A significant  
280 number of studies are dedicated to understanding EPS and SMP behavior and role in AnMBR  
281 fouling (Chen et al., 2017a; Xiong et al., 2016; Ding et al., 2015; Chen et al., 2014; Herrera-

282 Robledo et al., 2011). EPS and SMP play a significant role in membrane fouling due to  
283 surface blocking and pore accumulation. SMP which consists mainly of soluble proteins,  
284 polysaccharides and humic-like material is defined as an organic matter that can pass through  
285 a filter of 0.45  $\mu\text{m}$  and is responsible for adsorption on the surface and within the membrane  
286 pores (Chen et al., 2017a). The composition of extracellular polymeric substances (EPS)  
287 depends on the process parameters and wastewater composition as well as its origin.  
288 However, in general, it consists of insoluble carbohydrates, proteins, lipids and nucleic acid  
289 in a highly hydrated gel matrix (Laspidou et al., 2002). EPS contributes mainly to the cake  
290 layer fouling due to its ability to flocculate sludge on the membrane surface (Chen et al.,  
291 2017a). A study (Liu et al., 2012) explained that the cake formation in such systems was due  
292 to higher soluble microbial products (SMP) and higher tightly bound to loosely bound  
293 extracellular polymeric substances (EPS) ratio. Pore constriction and blockage results mostly  
294 in irreversible fouling but correspond only to 0.9% of total resistance to filtration, hence  
295 minimization of cake formation is more strategic than pore fouling in overall fouling  
296 mitigation (Chen et al., 2014).

297 Ding et al. (2015) reported that EPS extracted from bulk sludge and cake sludge were  
298 responsible for the largest decline in membrane flux. Herrera-Robledo et al. (2011) found that  
299 80% of the fouling was caused by EPS and SMP in PVDF membrane tubes. Shin et al. (2014)  
300 employed an intermittent membrane relaxation mechanism to control the fouling and drastic  
301 rise of TMP.

302 Fouling phenomenon is also influenced by the type of membrane modules in pilot-scale  
303 AnMBR. Flat membranes appear to be more sensitive to fouling than hollow fiber  
304 membranes. Indeed, it appears that scouring methods are less effective for FS than for HF  
305 and a higher fouling rate of 3.33 Pa/s was observed for Config-1, i.e., flat-sheet membrane

306 module (Martinez-Sosa et al., 2011); where a much lower fouling rate of 0-2.5 Pa/s was  
307 observed for Config-13, i.e., a hollow fiber membrane module (Martinez-Sosa et al., 2011).

308 Different fouling rates are observed also due to the difference in TMP. TMP of 17.7 kPa and  
309 10 kPa were observed for Config-1 and Config-10 respectively indicating higher fouling in  
310 Config-1. It is evident from TMP and fouling rate values that Config-1 is more affected by  
311 fouling than Config-10 due to the requirement of higher TMP to achieve the same permeate  
312 flux. A higher level of fouling rates needs higher energy to reduce it. Config-1 requires  
313 higher energy for fouling control (1.28 kWh/m<sup>3</sup>) when compared to Config-10 (0.23  
314 kWh/m<sup>3</sup>).

### 315 **2.3. Fouling mitigation strategies in pilot-scale AnMBRs**

316 Three major strategies for fouling mitigation are described below: scouring methods,  
317 filtration cycles and chemical cleaning cycles.

#### 318 **2.3.1. Scouring methods**

319 Three methods: biogas sparging (GSP), particle sparging (PSP) and rotating membrane  
320 (RMEM) can be used to provide shear stress to mitigate membrane fouling. GSP approach is  
321 the most common strategy to control fouling in AnMBR; this technique has been used in 11  
322 out of 13 pilot-scale studies referenced in Table 2. The crossflow velocity (CFV) is  
323 considered as a key factor for fouling mitigation when sludge recirculation is applied  
324 (Skouteris et al., 2012). Almost all the pilot-scale AnMBR systems (except GS-H5)  
325 incorporates sludge recirculation in the membrane tank to allow turbulent flow regime close  
326 to the membrane surface thereby reducing the thickness of the laminar boundary layer with  
327 better mixing of sludge (Lin et al., 2013; Smith et al., 2012). The applied CFV and Gas  
328 Sparging Intensity (GSI<sub>m</sub>) values were 9.7–94 m/h and 0.15–1.20 N.m<sup>3</sup>/(m<sup>2</sup>h), respectively.  
329 Config-7 used FeCl<sub>3</sub> (26 mg/L) for flux enhancement. Addition of coagulant results in the

330 reduction of fouling potential by enhancing floc sizes of the colloidal foulants present in the  
331 bulk, i.e., improvement of membrane fouling mitigation (Judd, 2011; Holbrook et al., 2004).

332 AnMBR with PSP (Config-13) is known as AFMBR (i.e., anaerobic fluidized membrane  
333 bioreactor). In AFMBR, fluidizing granular activated carbon (GAC) acts as the support  
334 medium for microorganisms along with playing a role in the scouring of the membrane  
335 surface (Kim et al., 2011). AFMBR requires a lower amount of energy and shows lower  
336 fouling potential as compared to AnMBR with gas sparging (Aslam et al., 2016; Shoener et  
337 al. 2016; Aslam et al. 2014).

338 Rotating membrane sparging is the last fouling control approach employed in Config-13 of  
339 pilot-scale AnMBR. This system makes use of the turbulence produced by the rotating  
340 membrane module to mitigate the foulant deposition on the membrane surface (Jiang et al.,  
341 2012). This technique has recently emerged as an efficient method with higher fouling  
342 control and lower energy consumption ( Ruigómez et al. 2017; Ruigómez et al. 2016;).

### 343 **2.3.2. Filtration cycles in AnMBRs**

344 All the above pilot-scale configurations except Config-13 were operated intermittently, i.e.  
345 filtration cycles were followed by relaxation cycles with optional cycles of backwashing.  
346 Most of the configurations used intermittent cycles of filtration with an integrated  
347 backwashing cycle except for the case of Config-12 and Config-7, respectively. Generally,  
348 the filtration time with respect to the total time of operation was larger with backwashing  
349 (around 79–96 %) than without backwashing (around 80–83 %).

### 350 **2.3.3. Chemical cleaning cycles**

351 A preventive approach to uphold the membrane permeability is intermittent maintenance  
352 chemical cleaning. Only 2 out of 13 experiments used chemical cleaning. In Config-7,  
353 maintenance cleaning was performed after every 7 days using a backwashing liquid

354 consisting of 2000 mg/L citric acid at a flux of 32.2 LMH. Config-7 also underwent recovery  
355 cleaning by soaking the membrane sequentially (for 16 hours) in the solution of 2000 mg/L  
356 of NaOCl and 2000 mg/L of citric acid, respectively. In Config-8, recovery cleaning was  
357 performed 7 times (for 4–6 hours) during a period of 1350 days with 1000 mg/L solution of  
358 NaOCl at room temperature.

#### 359 **2.4. Removal of organics in pilot-scale AnMBRs**

360 Operating conditions and performances of pilot-scale AnMBR used for DWWT are  
361 illustrated in Table 1 and Table 2. It was observed in all 13 studies that COD feed  
362 concentrations ranged from 198 mg/L to 1460 mg/L and HRT was in the range of 2.20–33  
363 hours. For WW having higher concentrations of COD in the influent (i.e., COD > 500 mg/L),  
364 HRT was generally higher than 10 hours. The maximum HRT of 33 hours was observed for  
365 Config-13 while the minimum HRT of 2.2 hours was observed for Config-6. SRT for all  
366 AnMBR configurations ranged from 6 days to infinite value (no waste of biomass, except for  
367 sludge sampling). The maximum SRT of infinite days was observed for Config-8 and  
368 minimum SRT of 6.2 days was observed for Config-12. On the other hand, the maximum  
369 Organic Loading Rate (OLR) range of 3.77–4.97 kg<sub>COD</sub>/(m<sup>3</sup>.d) was observed for Config-10  
370 while Config-3 showed the minimum OLR range of 0.3–1.1 kg<sub>COD</sub>/(m<sup>3</sup>.d).

371 The majority of the AnMBR systems were conducted under room temperature conditions  
372 (i.e., 17–35 °C) and Config-10 was operated over a broad range of temperatures (from 9 to 30  
373 °C). The operating permeation flux was within 4.1–17 LMH; the highest flux of 17 LMH was  
374 achieved after the incorporation of 26 mg/L of FeCl<sub>3</sub> to Config-7 and Config-10. The  
375 concentrations of MLSS ranged from 600 to 32000 mg/L. It must be noticed that MLSS was  
376 higher in GSP systems (around 7000–32000 mg/L) than in PS systems (around 600–12000  
377 mg/L).

378 All the configurations exhibited COD removal efficiencies greater than 85%. Config-12  
379 shows a high COD removal efficiency (more than 90%) even if this process was conducted in  
380 psychrophilic conditions (9–11 °C). Introduction of the flux enhancer (e.g., FeCl<sub>3</sub>) improved  
381 COD removal efficiency (79.9% to 93.7%) for Config-7 and Config-10 due to aggregation of  
382 colloidal organics followed by their rejection by the membrane.

## 383 **2.5. Energy requirements in the pilot-scale AnMBRs for fouling** 384 **mitigation**

385 An increasing crossflow velocity allows reducing membrane fouling but requires more  
386 energy consumption. For example, increasing crossflow velocity from 1 to 2 m/s increased  
387 permeate flux by about 20 %, but also the energy requirement by about 60 % (Bourgeois et  
388 al., 2001). Besides, the high shear rate can provide a negative effect on microbial activities in  
389 AnMBR due to the disaggregation of microbial flocs. The data of energy consumption along  
390 with the corresponding transmembrane pressure for all the AnMBR configurations (11 GSP  
391 AnMBRs, one PSP AnMBR and one RMEM AnMBR) is given in Table 3. It is evident from  
392 Table 3 that the energy requirement for fouling control in AnMBRs is the most significant  
393 constituent of the system, as it constitutes more than 75% of the total energy intake ( Smith et  
394 al., 2014; Lin et al., 2011). The energy requirement for fouling mitigation mainly relies on the  
395 scouring method (Pretel et al., 2014). The estimated energy demand in the GSP AnMBR  
396 system varies between 0.05 kWh/m<sup>3</sup> and 1.66 kWh/m<sup>3</sup> with an average energy of around 0.39  
397 kWh/m<sup>3</sup>. Among all the GSP AnMBRs, the highest energy demand was observed for the FS  
398 membrane (Config-1). The average energy demand for Config-1 was 1.66 kWh/m<sup>3</sup> having a  
399 peak GSI<sub>m</sub> value of 1.2 N m<sup>3</sup>/ (m<sup>2</sup>.h) with a comparatively lower critical flux of 7.0 LMH  
400 (Krzeminski et al., 2012; Verrecht et al., 2008). The average energy requirement for GSP  
401 AnMBRs with HF membranes (Config-2 to Config-10) was 0.28 kWh/m<sup>3</sup>. However, GSP



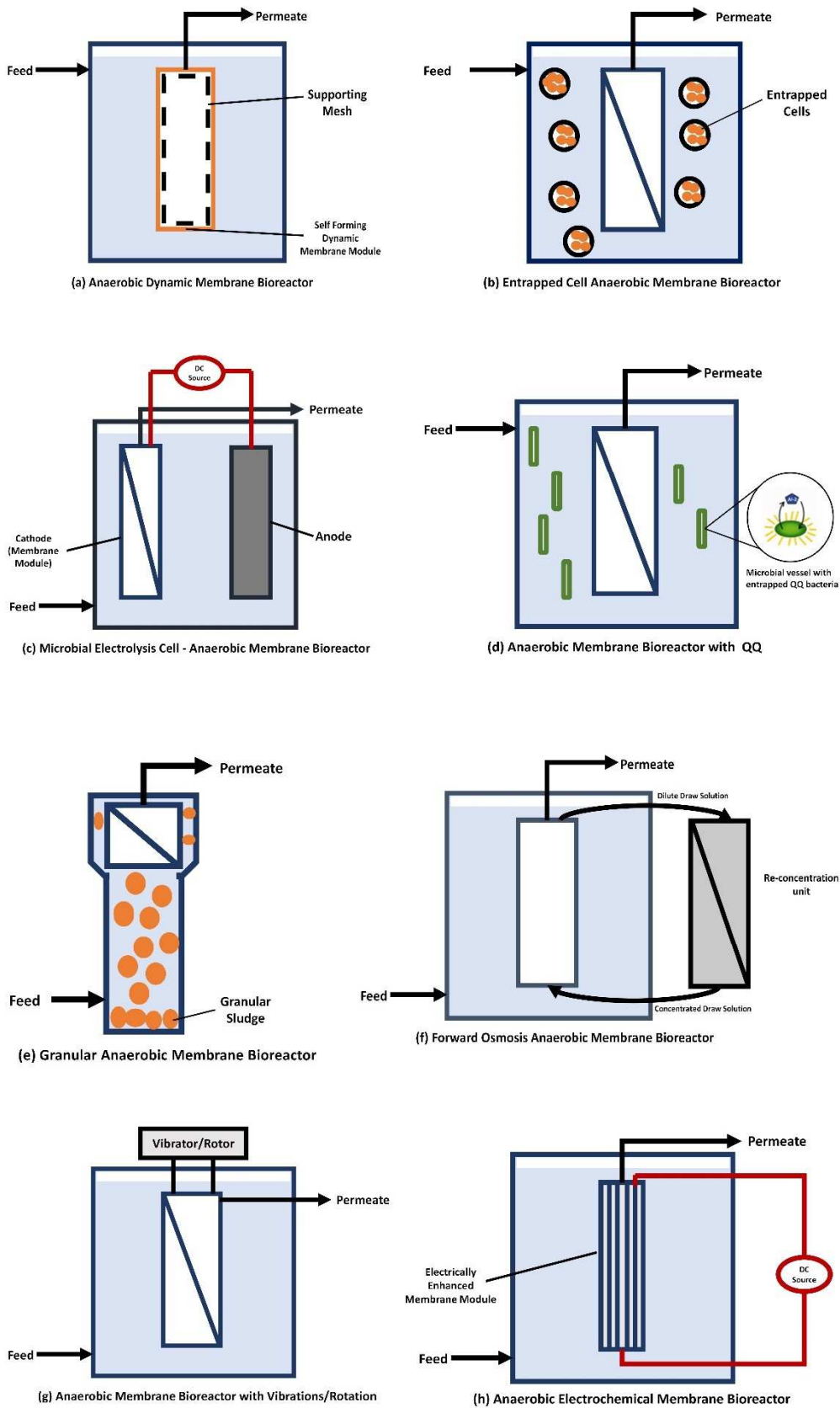
402 AnMBRs still requires roughly 65% less energy than AeMBRs (0.70–0.90 kWh/m<sup>3</sup>)  
403 (Krzeminski et al., 2012).

404 The energy requirement for a GSP AnMBR depends on the flux conditions and GSI<sub>m</sub>. HF  
405 AnMBRs require lower GSI<sub>m</sub> than FS modules. Amongst hollow fiber AnMBRs, Config-7,  
406 Config-9 and Config-10 required the lowest energies due to lower GSI<sub>m</sub> values of around  
407 0.15-0.32 N.m<sup>3</sup>/(m<sup>2</sup>.h). For Config-7 and Config-10, lower GSI<sub>m</sub> was achieved by the  
408 addition of coagulant (i.e., flux enhancer). On the contrary, lower GSI<sub>m</sub> in Config-9 is due to  
409 lower solid concentration in the membrane containing tank coupled with recirculation of  
410 liquid between the membrane tank and the biological reactor.

411 The relationship between energy requirement and flux is illustrated by drawing a comparison  
412 of flux, TMP and energy demands for Config-3 and Config-4 operated under the same GSI<sub>m</sub>.  
413 The increment of flux from 10 LMH (Config-3) to 13.3 LMH (Config-4) at the same GSI<sub>m</sub>  
414 resulted in lower energy demand (0.164 kWh/m<sup>3</sup>) for Config-4 as compared to Config-3  
415 (0.198 kWh/m<sup>3</sup>). Similarly, the specific energy demand of PSP AnMBR can also be reduced  
416 by increasing the flux. The energy demand in the PSP AnMBR (Config-10) for GAC  
417 fluidization was reported to be 0.102 kWh/m<sup>3</sup>. McCarty et al. (2011) assessed the decrease in  
418 energy requirement to be 0.070 kWh/m<sup>3</sup> for PSP AnMBR. The estimated energy demand for  
419 a staged anaerobic fluidized AnMBR (Config-10) is 0.23 kWh/m<sup>3</sup> with a flux of 10 LMH  
420 and membrane rotation velocity of 100 rpm.

### 421 **3. Novel AnMBRs for fouling mitigation**

422 This section has designed to demonstrate (1) novel AnMBRs configurations and their impact  
423 on fouling; (2) impact of specialized fouling mitigation techniques on total fouling control in  
424 AnMBR for DWWT. Figure 1 shows the schematics of different novel configurations (a, c, e  
425 and f) of AnMBRs and specialized fouling mitigation strategies (b, d, g and h).



426

427

428

429

**Figure 1** Schematics of different AnMBR schemes. (a) Dynamic AnMBR (b) Entrapped Cell AnMBR (c) MEC-AnMBR (d) AnMBR with QQ (e) G-AnMBR (f) FO-AnMBR (g) AnMBR with vibration/rotation (h) Electrochemical AnMBR

### 430        **3.1. Granular-AnMBR**

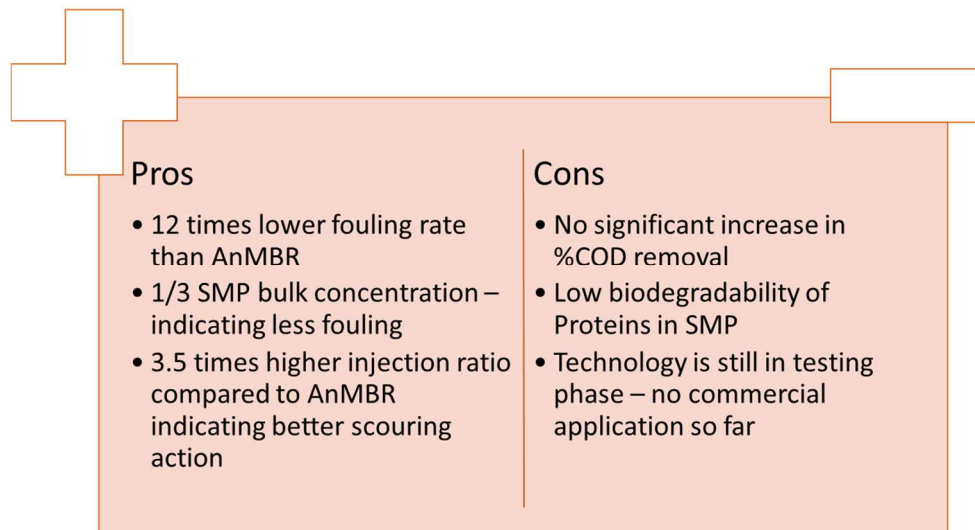
431        Coupling of membrane separation with anaerobic granular sludge (AnGS) reactors constitutes  
432        an interesting perspective. The most promising feature in G-AnMBR is the way to control  
433        membrane fouling. The structure of AnGS is not only justified for decantation but also to  
434        limit fouling phenomena. AnGS recirculation promotes mechanical scouring actions  
435        alongside the membrane surface and contributes to reduce membrane fouling. Fluidized  
436        AnGS comes into physical contact with the membrane surface, and their physical movement  
437        helps to reduce membrane fouling at a relatively low energy cost (Kim et al., 2011).  
438        Furthermore, when AnGS are used as fluidized media, the biological kinetics rate increases  
439        leading to a reduction of colloidal and dissolved organic matter (DOM), further decreasing  
440        irreversible fouling due to this DOM and increasing treatment stability and efficiency (Aslam  
441        et al., 2014). However, very little knowledge is available regarding the mechanisms of the  
442        mechanical cleaning process and fouling mitigation during DWWT by AnGS in G-AnMBR.  
443        Although G-AnMBR has a great potential to achieve energy-positive domestic wastewater  
444        treatment, the extent of fouling and energy consumption in the G-AnMBR can be strongly  
445        affected by the biological and physio-chemical properties of the AnGS. There are still several  
446        research questions to be resolved to optimize the selection of granular sludge. Besides, the  
447        energy requirement for fluidization in the G-AnMBR is closely related to the packing ratio of  
448        AnGS and their physical properties such as nature, size and shape. A better understanding of  
449        the links between AnGS characteristics, operating conditions (characteristic of the feed  
450        (DOM, mass loading rate), hydrodynamics, solid and liquid residence time) and fouling  
451        mitigation efficiencies (TMP, resistances), are thus needed (Aslam et al., 2017; Choo et al.,  
452        1998). The following section will discuss studies on G-AnMBR in detail and will also present  
453        a comparison with conventional AnMBR.

### 454 **3.1.1. G-AnMBR v/s Conventional AnMBR**

455 G-AnMBR combines the benefits of granulation and membrane separation and overcome  
456 drawbacks associated with conventional anaerobic sludge bioreactor like long startup time,  
457 high operating temperature, poor settling, poor nutrient removal and need for post-treatment  
458 (Chen et al., 2016). A study (Martin-Garcia et al., 2011) comparing the performance of G-  
459 AnMBR versus conventional flocculated AnMBR during 250 days showed almost similar  
460 COD removal in both the cases (84% in AnMBR and 86% in G-AnMBR). However, a great  
461 difference in MLSS and SMP bulk concentrations ( $598 \pm 150$  mgCOD/L for AnMBR and  
462  $198 \pm 73$  mgCOD/L for G-AnMBR) was found, indicating less fouling propensity in G-  
463 AnMBR. Protein to carbohydrate (P/C) ratio in SMP was found higher for G-AnMBR (2.8 in  
464 G-AnMBR v/s 2.3 for AnMBR), probably due to infinite SRT compared to AnMBR which  
465 resulted in accumulation of proteins in SMP and thus indicating low biodegradability of  
466 proteins in case of G-AnMBR.

467 Superficial liquid velocity ( $V_L$ ) induced by gas velocity ( $V_G$ ) (nitrogen in this case) was much  
468 higher in the case of G-AnMBR for similar  $V_G$  of 0.02 m/s, (i.e. had  $V_L$  of 0.26 and 0.09 m/s  
469 for G-AnMBR and AnMBR respectively). Thus, injection ratio ( $IR = V_G / (V_G + V_L)$ ) was  
470 much lower for G-AnMBR (0.07 to 0.2) compared to AnMBR (0.26 to 0.71), indicating  
471 better scouring action in the case of G-AnMBR. Interestingly, for the same flux of 11 to 12  
472 LMH, the fouling rate in G-AnMBR was only 1.67-3.33 Pa/s compared to a much higher  
473 fouling rate of 13.33 to 41.67 Pa/s in AnMBR. Also, using fluid pumping to generate  
474 crossflow velocity (CFV) for the same increase in CFV (0.4 to 2m/s), a greater increase of  
475 flux was observed in G-AnMBR (4 to 41 LMH) compared to AnMBR (4 to 19 LMH)  
476 indicating lesser CFV requirements for suppression of cake enhanced concentration  
477 polarization in case of G-AnMBR. Even in the case of an immersed HF module, gas  
478 (nitrogen) demand was 50% less in G-AnMBR as compared to AnMBR (Martin-Garcia et al.,

479 2011). These low scouring requirements and less fouling make G-AnMBR a potential  
480 solution for fouling and energy-related issues in AnMBR for DWWT. Figure 2 summarizes  
481 the pros and cons of G-AnMBR.



482

483

*Figure 2 Pros and Cons of G-AnMBR compared to AnMBR*

484

### **3.1.2. Effect of configuration on fouling in G-AnMBR**

485 When internally submerged (SG-AnMBR) and external (EG-AnMBR) membrane

486 configurations were compared, it was found that with approximately the same COD removal

487 efficiencies SG- AnMBR was more prone to fouling than EG-AnMBR (Chen et al., 2017b).

488 It was explained that the direct addition of membrane in the reactor results in almost

489 complete colloidal flocs retention resulting in their accumulation and thus enhancement in the

490 growth of bulking sludge which is dispersed and has poor immobilization. This phenomenon

491 dominates over the growth of granular sludge and thus resulted in low biomass growth rate

492 (0.02 gMLSS/d v/s 0.05 gMLSS/d), settling velocity (12.1-17.2 m/h v/s 14.1-28.5 m/h) and

493 zeta-potential (-19.1 v/s -13.1) in SG-AnMBR compared to EG-AnMBR. Accumulation of

494 fine and small flocs in SG-AnMBR also accounted for higher MLSS ( $180.2 \pm 9.12$  mg/L)

495 which in turn was responsible for higher EPS and SMP production thus more fouling.

496 Moreover, protein-based EPS in the cake layer was higher for SG-AnMBR (11.7 mg/g of

497 cake layer) than EG-AnMBR (8.5 mg/g of cake layer), indicating a stickier cake layer  
498 resulting in more severe fouling and permeate flux drop. The same was observed for SMP  
499 (18.6 v/s 10.2 mg/g of cake layer) resulting in more adhesion of sludge on the membrane  
500 surface. This could cause irreversible fouling by forming a thin gel layer at the membrane  
501 surface (Chen et al., 2017b). Biopolymers were found to be major contributors in organic  
502 fouling with a noticeable difference of concentration in both configurations. Reported values  
503 in SG-AnMBR and EG-AnMBR were 14.6 and 6.8 mg/L, respectively. Their high  
504 concentration might result in the build-up of a hydrophilic layer by biopolymers attachment  
505 on the membrane surface. Building blocks (BB), low molecular weight (LMW) neutrals and  
506 acidic compounds can also enhance biopolymer production and attachment to the membrane  
507 surface thereby contributing to the increased fouling. Their concentrations were also higher  
508 for SG-AnMBR (6.3 mg/L BB and 9.7 mg/L LMW) compared to EG-AnMBR (4.6 mg/L BB  
509 and 5.9 mg/L LMW) (Chen et al., 2017b).

### 510 **3.1.3. Effect of gas sparging regime on performance and energy demand in G-** 511 **AnMBR**

512 To evaluate the effect of the GSP regime on performance and energy demand of G-AnMBR,  
513 (Wang et al., 2018a), a study was performed with pilot-scale SG-AnMBR treating DWW and  
514 operating it under three different GSP conditions that were: (1) continuous filtration and GSP,  
515 (2) continuous filtration with intermittent GSP and (3) intermittent filtration and GSP in  
516 pseudo dead-end filtration (filtration cycle of nine minutes without GSP followed by one  
517 minute of GSP and membrane relaxation). HRT was kept at 8 hours and the temperature of  
518 the influent was around  $16.3\pm 3.7$  °C. 40 % bed expansion was achieved by maintaining an  
519 up-flow velocity of 0.8-0.9 m/h. During the 400 days trial, there was no withdrawal of  
520 biomass. Intermittent filtration and sparging in pseudo dead-end filtration was found to be the  
521 most suitable configuration among all three that had provided sustainable fouling rates with

522 minimum energy requirements ( $0.133 \text{ kWh/m}^3$  with net SGD of  $0.2 \text{ m}^3/(\text{m}^2\cdot\text{h})$ ). The authors  
523 explained that in the case of dead-end filtration, particulate materials, colloids and soluble  
524 materials deposited simultaneously compared to continuous GSP where induced shear stress  
525 close to the membrane surface caused particle size segregation resulting in preferential  
526 migration of colloids towards the membrane surface. This deposition resulted in a  
527 heterogenous cake layer. However, this is only possible when solid concentration is low  
528 enough to allow limited cake deposition in a specific filtration cycle since TMP and filtration  
529 time influence cake compressibility. At 13.5 LMH, SGD per unit permeate of  $14.8 \text{ m}^3/\text{m}^3$  was  
530 similar to that in a plant scale AeMBR treating municipal waste ( $14\text{-}30 \text{ m}^3/\text{m}^3$ ) despite more  
531 complex fouling behaviors on anaerobic treatment.

#### 532 **3.1.4. Sponge assisted G-AnMBR**

533 To minimize the fouling rate in SG-AnMBR, the effect of polyurethane sponge incorporation  
534 was studied (Chen et al., 2017c). Decreasing the fouling rate in SG-AnMBR will make it  
535 superior to external configuration since it has lower capital and operational costs. Sponges act  
536 as media to provide a high specific area for the growth of biomass by increasing the  
537 flocculation of sludge due to porosity and resistance against hydrolysis. Sponge assisted G-  
538 AnMBR contains about 84 % of total granular sludge and this is about two times higher than  
539 conventional granular bioreactor (about 42.5 %) due to the immobilization of fine particles on  
540 or inside the pores of the sponge. EPS plays an important role in granular formation by  
541 allowing cell integration and helping to keep sludge intact. The average fouling rate in  
542 sponge assisted G-AnMBR was about  $0.006 \text{ Pa/s}$  which is lower than that of conventional G-  
543 AnMBR ( $0.014 \text{ Pa/s}$ ). This difference in fouling rates can be explained by SMP and EPS  
544 concentrations in mixed liquor as well as the cake layer. In conventional G-AnMBR, granules  
545 and floc breakage increase SMP concentration ( $47.3\pm 7.6 \text{ mg/L}$ ) and EPS ( $24.5\pm 11 \text{ mg/L}$ ). In  
546 contrast, sponge assisted G-AnMBR exhibits relatively low SMP and EPS concentrations of

547 15.9±3.5 mg/L and 17±6.2 mg/L, respectively. Lower SMP and EPS concentrations are due  
548 to adsorption of sludge on the sponge and the majority of biodegradation occurs inside  
549 granules and sponge-attached biomass. Reduced free fine flocs and colloids in sponge  
550 assisted G-AnMBR resulted in a less dense fouling cake layer than that observed in  
551 conventional G-AnMBR. Overall fouling resistance is reduced since cake layer fouling is  
552 dominant in AnMBR. Moreover, cell lysis in conventional G-AnMBR results in higher  
553 protein EPS (12.1 mg/g of cake layer in conventional v/s 10.7 mg/g of cake layer in sponge  
554 assisted) and protein SMP (8.2 mg/g of cake layer in conventional v/s 5.6 mg/g of cake layer  
555 in sponge assisted) which forms stickier cake and thus contributes more to fouling filtration  
556 resistance. In addition to less fouling, sponge assisted G-AnMBR also provides the benefit of  
557 a 17 % higher methane yield. Another configuration that has employed sponges and that was  
558 designed for fouling control was sponge assisted AnMBR with rotary disk (Kim et al., 2014).  
559 It utilized polyurethane sponge to support microbial growth and the disk rotates parallel to the  
560 surface of 2 flat sheet submerged membrane for fouling control. 96% COD removal and  
561 150±29 mL CH<sub>4</sub>/g COD methane yield were obtained by performing experiments using a 50  
562 % volume fraction of sponge, 6 hours HRT, 11 LMH flux and 70 rpm rotation speed of the  
563 disk. TMP rise rate of 0.15 Pa/s was observed compared to 5 Pa/s without rotation indicating  
564 effective fouling mitigation. This was attributed to high collision energy between sponge and  
565 membrane surface, reducing cake enhanced concentration polarization. Operational  
566 electricity consumption was ten times lower by combining rotation and sponge media than  
567 the energy produced by methane combustion. As a result, the operation of this hybrid  
568 AnMBR provided net positive energy of 0.04 kWh/m<sup>3</sup>. This study demonstrated a potential  
569 configuration for the net positive energy operation of AnMBR for DWWT. However, a  
570 detailed study regarding fouling behaviors under different operational conditions and floating  
571 media is necessary before scale-up.



### 572 **3.2. Entrapped cell AnMBR**

573 Cell entrapment makes use of a polymer matrix to artificially entrap cells providing increases  
574 resistance to washout. Juntawang et al. (2017) used phosphorylated polyvinyl alcohol for cell  
575 entrapment in AnMBR and compared it with conventional suspended cell AnMBR. Both  
576 units showed the same COD removal efficiencies (approximately 84 % COD removal) which  
577 indicates that cell entrapment has no negative impact on treatment. Fouling resistance in  
578 entrapped cell based AnMBR was found to be  $0.32 \times 10^6$  1/m for pore blockage and  $1.06 \times 10^6$   
579 1/m for the cake layer. These are less than the suspended one ( $2.54 \times 10^6$  1/m for pore  
580 blockage and  $1.69 \times 10^6$  1/m for cake layer) indicating less fouling propensity. Cell entrapment  
581 results in lower EPS and SMP concentration as well as larger particle size. Conventional  
582 AnMBR has much higher pore blockage resistance which indicates more potential for  
583 irreversible fouling and thus would need more intensive chemical cleaning. However,  
584 bacterial communities in both the cases were similar except bacteroidetes colonies which  
585 were more abundant in suspended cell AnMBR than in entrapped cell AnMBR. The  
586 formation of these colonies is favored in the presence of proteinaceous EPS and thus high  
587 protein to carbohydrate ratio of EPS. Methane yields in both cases were almost similar i.e.  
588 0.23 L/d for entrapped cell bases AnMBR and 0.28 L/d for the suspended one.

589 In 2019, Juntawang et al. (2019) have used the entrapment technique in FO-AnMBR.  
590 Polyvinyl acetate (PVA) based entrapped cells were used in side-stream configuration with  
591 thin-film composite (TFC) FO membrane to treat domestic wastewater having initial soluble  
592 COD of 542 mg/L. Two draw solutions namely NaCl (1.5M) and  $(\text{NH}_4)_2\text{SO}_4$  (1M) were  
593 tested. In the case of entrapped cells, flux decline rates were 0.042 and 0.049 LMH/day for  
594 FO-AnMBRs using NaCl and  $(\text{NH}_4)_2\text{SO}_4$  as draw solutions, respectively. Whereas, for  
595 suspended cell FO-AnMBR they were 0.057 and 0.074 LMH/day, respectively. Additionally,  
596 EPS was 35% and 13% less and SMP was 65% and 68% less in entrapped cell FO-AnMBR

597 using NaCl and  $(\text{NH}_4)_2\text{SO}_4$  as draw solutions, respectively. This could be explained by the  
598 fact that entrapment keeps the cell activity restricted, and thus control the formation of EPS  
599 and SMP. In addition, it might also keep EPS and SMP trapped in, suggesting that fouling  
600 potential should be higher with suspended cells than entrapped cells. Entrapped cell-based  
601 technologies could be a step forward in terms of fouling mitigation. As seen, they provide  
602 better fouling mitigation than conventional AnMBR. More research in this area is needed to  
603 conduct energy and cost comparisons so that the technique can be further developed.

### 604 **3.3. Dynamic AnMBR**

605 In the recent years, anaerobic dynamic membrane bioreactors (AnDMBR) have been  
606 investigated as a sustainable solution to wastewater treatment due to their low cost of the  
607 membrane, reasonable treatment efficiency (providing 60-90% COD removal and 90-100%  
608 turbidity and suspended solids removal) and less fouling ( $0.6 \times 10^9 \text{ m}^{-1}/\text{h}$  rate of increase of  
609 fouling resistance compared to  $40 \times 10^9 \text{ m}^{-1}/\text{h}$  in AnMBRs) (Hu et al., 2018a). Dynamic  
610 membrane (DM) bioreactors make use of cheap materials like meshes or fiber cloth as  
611 support on which the cake layer is formed instead of expensive ultrafiltration or  
612 microfiltration membranes. This cake layer acts as an additional or secondary membrane  
613 (dynamic membrane layer) due to its capability of rejecting pollutants such as colloidal  
614 materials, microbial cells and organics. Although DM was reported firstly by workers of Oak  
615 Ridge National Laboratories in 1965, its application in AnMBR is relatively new and less  
616 developed. (Alepu et al., 2016). This section aims at reviewing the existing literature on  
617 AnDMBR and how they are useful in resolving the fouling issues for domestic wastewater  
618 treatment.

#### 619 **3.3.1. DM layer formation mechanism in AnDMBR**

620 A study (Zhang et al., 2010) proposed a three-step formation mechanism that involves a  
621 separation layer formation stage, stage of stable growth and finally a fouling stage. This

622 mechanism was confirmed by other studies as well ( Hu et al., 2018b; Siddiqui et al., 2018;  
623 Alepu et al., 2016; Alibardi et al., 2016; Ersahin et al., 2016; Zhang et al., 2011). The time  
624 required for a stable DM layer formation depends on the operation mode of the reactor. In  
625 gravity-driven mode, which allows continuous extraction of effluent without relaxation, this  
626 time is about 40-100 minutes whereas in the pressure-driven mode it is about 10-25 days.  
627 Operating with intermittent permeate production leads to longer stabilization time both in  
628 pressure-driven mode (approximately 50 days) and gravity-driven mode (7 days) (Hu et al.,  
629 2018a) compared to the continuous operation.

### 630 **3.3.2. Submerged versus external dynamic AnMBR**

631 Membrane configuration plays a very significant role in defining performance efficiency as  
632 well as fouling propensity in AnDMBR. Submerged AnDMBR was found to be slightly  
633 better in terms of removal efficiency on COD and turbidity (99.5 and 99.7 %) than external  
634 configuration (Ersahin et al., 2017). The authors also found out that the time required to form  
635 a stable DM layer in submerged configuration was only 10 days while it was 20 days in  
636 external configuration at the same permeate flux of 2.2 LMH. The DM layer was less stable  
637 in the external dynamic membrane due to the high shear rate which affected the system  
638 performance. These results indicate that submerged configuration is more appropriate than  
639 the external configuration for applications that require smaller startup time and a more stable  
640 DM layer. It was also mentioned that increased shear stress in the external case disturbed the  
641 balance of the microbial ecosystem and reduced the methane formation thus providing a  
642 negative impact on the overall energy balance of the system. This was proved by the fact that  
643 the submerged configuration had an average methane flow of 2.4 L/days whereas the external  
644 one had 1.9 L/days. However, it was also seen that the submerged configuration has higher  
645 filtration resistance ( $10.2 \times 10^{16}$  1/m) and TMP (68.0 kPa) than the external one with  $7.4 \times 10^{16}$   
646 1/m resistance and 38.0 kPa TMP (Ersahin et al., 2017). Higher resistance in submerged

647 dynamic AnMBR was probably caused by the formation of a thick DM layer. External  
648 recirculation has an additional scouring effect on the membrane. Differences in performance  
649 efficiencies were not too high. To understand which configuration is better for full-scale  
650 implementation, it would be important to establish complete energy and economical balances.

### 651 **3.3.3. Effect of operating conditions in AnDMBR**

652 Operating conditions have a significant effect on DM formation, performance and fouling in  
653 AnDMBR.

#### 654 **3.3.3.1. Effect of flux**

655 Higher operating fluxes are required in AnDMBR operations to reduce HRT and thus capital  
656 cost and footprint. It was observed that higher flux led to a faster formation of the DM layer.  
657 However, the time required for stable operating was reduced from 10 to 5 days as flux  
658 increased from 50 to 150 LMH. Higher flux resulted in a faster adsorption rate of suspended  
659 solids on the membrane surface (Wang et al., 2018). However, as for other membrane  
660 processes, higher fluxes can result in more severe fouling; Other studies observed that higher  
661 initial flux causes the higher increase of TMP (Siddiqui et al., 2018; Quek et al., 2017;  
662 Saleem et al., 2016; Chu et al., 2014).

#### 663 **3.3.3.2. Effect of organic load and sludge recycling**

664 Hu et al., (2018b) studied the effect of organic load and sludge recycling on the performance  
665 of a flat-sheet AnDMBR at 20 °C and a short HRT of 8 hours. In both cases, without sludge  
666 recycling (CFV=0.24 m/h) and with sludge recycling (CFV=0.83 m/h), when organic loading  
667 rate (OLR) was increased from 0.88 kg<sub>COD</sub>/(m<sup>3</sup>.d) to 3.01 kg<sub>COD</sub>/(m<sup>3</sup>.d) an increase in COD  
668 removal efficiency (approximately 23 % increase) as well as methane yield (approximately 5  
669 times increase) were observed. Higher COD removal was attributed to higher retention of  
670 organic compounds by a thicker DM layer. However, when OLR was increased, by adding  
671 synthetic wastewater, the availability of organic matter to bacteria was increased thus

672 allowing to produce more methane. With sludge recycling, CFV could be increased but it  
673 impacted the COD removal negatively. Similar results were observed by (Siddiqui et al.,  
674 2018) where an increase of CFV from 54 m/h to 90 m/h s increased turbidity in the permeate  
675 (from 10 to 17 NTU). Higher CFV has reduced the thickness of the DM layer, allowing  
676 particulate matter to pass through the membrane, which has led to lower permeate quality.  
677 The authors also observed that when the organic load was increased, the filtration properties  
678 of the membrane deteriorated. TMP did not exceed 15 kPa throughout the operational period  
679 when OLR was 0.88 kg<sub>COD</sub>/(m<sup>3</sup>.d), but it rose quickly to 20kPa within 20 d of operation for  
680 OLR of 1.5 kg<sub>COD</sub>/(m<sup>3</sup>.d). For the OLR of 3.01 kg<sub>COD</sub>/(m<sup>3</sup>.d), TMP was over 15 kPa in less  
681 than 20 d. This behavior could be explained by the fact that more biomass is accumulated in  
682 the DM layer at higher OLR.

683 The thickness of the DM layer with sludge recycling was less than without recycling (0.54-  
684 0.95 mm compared to 0.76-1.27mm without recycling) due to improved back transport by  
685 increased CFV. However, TMP increased faster (15-18 kPa with OLR of 0.88 kg<sub>COD</sub>/(m<sup>3</sup>.d))  
686 because of sludge structure modification during recirculation. Sludge recycling promoted the  
687 formation of fines and colloids, which may have accelerated pore blockage. Moreover, higher  
688 amounts of EPS and SMP were observed with sludge recycling (27.5 mg/g MLSS total EPS  
689 and 2.67 mg/g MLSS total SMP) compared to without sludge recycling where 13.58 mg/g  
690 MLSS total EPS and 1.06 mg/g MLSS total SMP was observed. They were responsible for  
691 the higher fouling rate in the case of sludge recycling.

#### 692 **3.3.3.3. Effect of operating time**

693 Operating time or filtration period can have a significant impact on the fouling rate in  
694 AnDMBR. It was observed that increasing filtration duration from 60 minutes to 720 minutes  
695 increased DM thickness from 2.29 mm to almost 3 mm. This results in irreversible fouling  
696 that could not be recovered by backwashing Moreover, the total amount of EPS increased

697 from 28 mg/g VSS to 42 mg/g VSS and total SMP increased from 4 mg/g VSS to 6.5 mg/g  
698 VSS as the operation time increased from 60 to 720 minutes (Siddiqui et al., 2018).

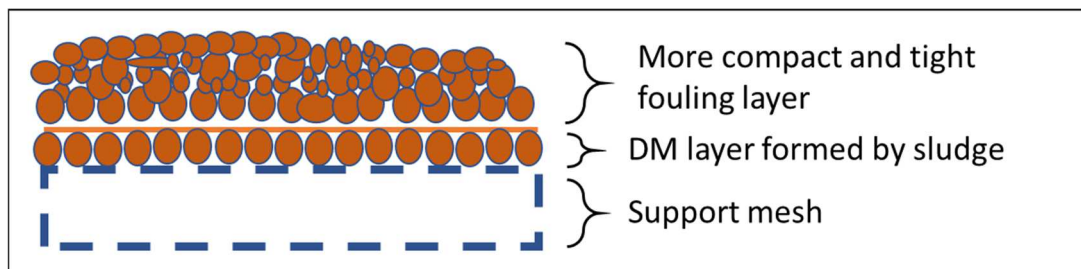
#### 699 **3.3.3.4. Effect of mesh pore size**

700 Mesh size from 10  $\mu\text{m}$  to 200  $\mu\text{m}$  was tested in AnDMBR studies. It can be observed (Table  
701 4), that an increase in mesh pore size improves the flux due to reduced filtration resistance.  
702 However, it affects the removal efficiency negatively by allowing more passage of solids  
703 through mesh pores than UF/MF membranes with much smaller pore sizes. Thus, a trade-off  
704 exists between permeate flux and removal efficiency. However, the difference in COD  
705 removal efficiency is not significant whereas with larger pore size (46  $\mu\text{m}$  pore size  
706 compared to 28  $\mu\text{m}$ ) filtration time can be increased up to four times indicating that smaller  
707 pore sizes are more prone to fouling (Quek et al., 2017). Moreover, meshes with larger pore  
708 sizes are cheaper than smaller pore size ones (Alibardi et al., 2016).

#### 709 **3.3.4. Fouling mechanism in AnDMBR**

710 Fouling has a different meaning for AnDMBRs. Initial accumulation of material on the mesh  
711 is the desired DM formation and after DM formation, subsequent deposition that leads to  
712 increased resistance to filtration is regarded as fouling (Chu et al., 2014). In one study (Wang  
713 et al., 2018) the membrane fouling mechanism for self-forming AnDMBR at 25 °C with  
714 polyamide nylon mesh (150  $\mu\text{m}$  pore size) supported by a hollow cylinder was described. The  
715 author experienced a three-stage fouling mechanism in AnDMBR operating at a flux of 100  
716 LMH. At the first stage, a decrease in resistance was observed. This was due to the increase  
717 in membrane hydrophilicity. Hydrophilicity reduces the adsorption of pollutants on the  
718 membrane surface. In the second stage, the resistance was rather stable due to the low content  
719 of EPS in the system. Finally, in the third stage, resistance increased sharply indicating  
720 blockage of membrane pores and enhanced compactness of cake. Zhang et al., (2011)  
721 observed membrane foulants in AnDMBR and observed a two-layered structure with an inner

722 layer tightly bonded to the mesh surface and an outer layer loosely bonded to the inner layer.  
723 SEM analysis showed that the cake layer developed during the first stage had larger pores  
724 and loosely packed sludge flocs. As the stages process, cake compactness has started to  
725 increase resulting in a very compact cake layer in the third stage. A similar observation was  
726 made by another study (Sun et al., 2018). It is assumed that solid accumulating varies in size,  
727 initial coarse particles get deposited but as the DM layer formed, the deposition of small  
728 particles leads to a denser cake (Figure 3). Moreover, EPS, polysaccharides, proteins and  
729 total cells (that are responsible for fouling in AnMBRs) increased in content as the stages  
730 proceed. It was also found (Ersahin et al., 2016) that the average amount of SMP, EPS and  
731 EPS P/C ratio of the DM layer was 21.5, 5.8 and 1.9 times (respectively) more than the bulk  
732 sludge indicating more resistance and stickier cake.



733

734 *Figure 3 Foulant layer distribution in AnDMBR*

### 735 **3.3.5. Fouling mitigation in AnDMBR**

736 AnDMBR has provided a cheaper alternative to expensive MF/UF membranes for AnMBRs.  
737 The buildup of cake is useful for AnDMBRs but beyond a certain level, it is regarded as  
738 fouling and must be controlled to have a long and stable operation. Fouling mitigation  
739 techniques like CFV, biogas sparging, back flushing and relaxation can serve the purpose.  
740 CFV could help in prolonging the AnDMBR operation however once the cake layer is  
741 developed application of large CFV, only has a short-term impact on fouling mitigation. It  
742 reduced the TMP value to half (from above 20 kPa to below 10 kPa) when CFV increased  
743 from 1 to 5 m/h (Alibardi et al., 2014) but this effect did not last for long and a sharp increase

744 in TMP was observed again after a short time. Moreover, high CFV could modify sludge  
745 resulting in a negative impact on filterability as well as biological activity (Hu et al., 2018b).

746 Some studies employed GSP as fouling control in AnDMBR (Li et al., 2017; Yang et al.,  
747 2017; Li et al., 2016). Application of biogas sparging (at a rate of 1 L/min) decreased foulants  
748 accumulation (38 days to reach maximum TMP with biogas sparging compared to 15 days  
749 without) on the mesh surface and offered a less rapid increase of TMP compared to the  
750 operation without biogas sparging. This indicates that biogas sparging can effectively scour  
751 the foulant layer in DM membranes and increase the operation time. A similar observation  
752 was made by Yang et al., (2017) that used N<sub>2</sub> sparging. However, it is important to control  
753 the sparging rate. Too much sparging can disturb the DM layer and affect the performance of  
754 AnDMBR. Compared to conventional AnMBR less sparging rate is required in AnDMBR as  
755 removal of foulants from mesh is easier than MF/UF (Li et al., 2016).

756 As AnDMBR makes use of the cake layer for filtration with comparatively cheap mesh  
757 supports which can be easily cleaned and the cake layer can be reformed, not much attention  
758 is given to in situ fouling control in AnDMBR. Hence there is a need to investigate in further  
759 detail the effect of in situ fouling control methods on performance as well as the lifetime of  
760 AnDMBRs. It is also needed to carry out economic and energy calculations on AnDMBR  
761 with and without in situ fouling control to establish comparisons with conventional AnMBRs  
762 and evaluate the feasibility of AnDMBRs for the long term and large-scale applications.

### 763 **3.4. Forward Osmosis AnMBR (FO-AnMBR)**

#### 764 **3.4.1. Configuration and working principle of FO-AnMBR**

765 FO-AnMBR combines an AnMBR where the membrane is a FO membrane instead of  
766 UF/MF in the conventional AnMBR process. FO relies on an osmotic gradient as a driving  
767 force; and therefore, high osmotic pressure solution (seawater, seawater brine, fertilizer...)

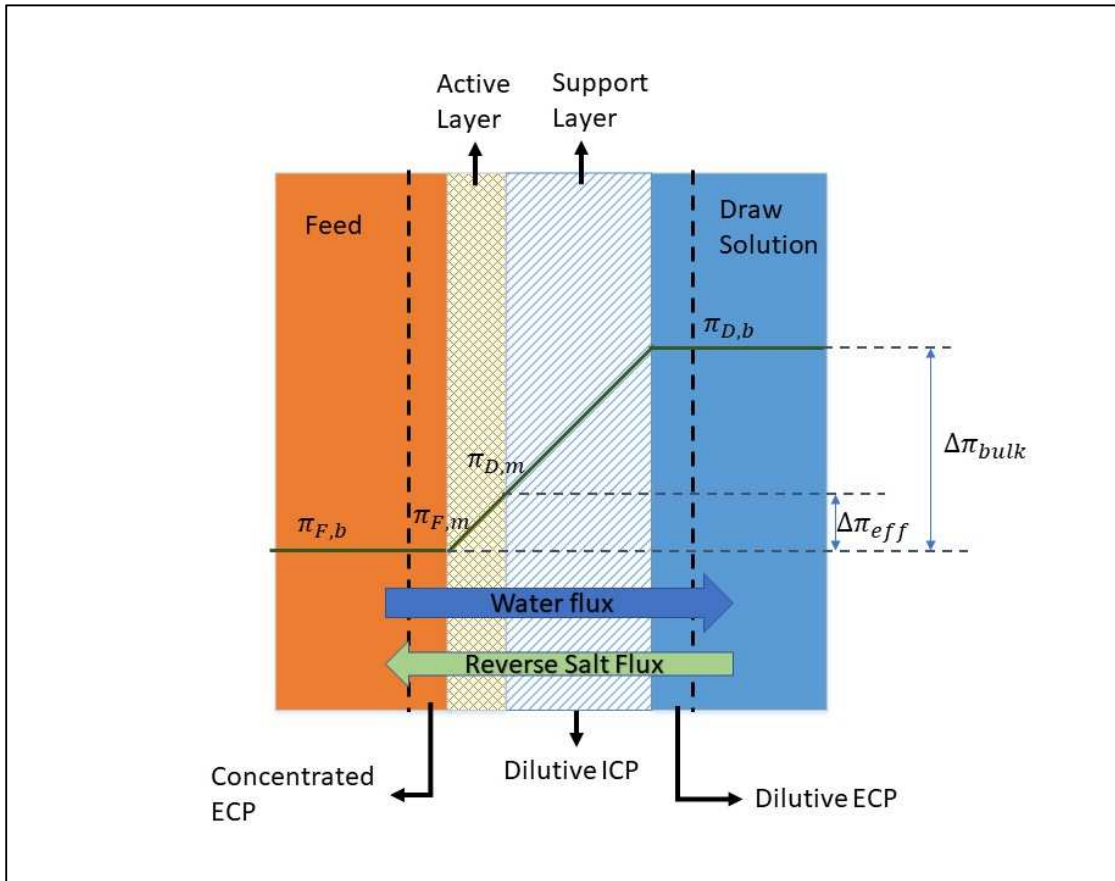


768 also called draw solution is used to allow for permeation and extraction of water from WW  
769 (feed) through the FO membrane. As such, unlike conventional AnMBR, FO-AnMBR  
770 operates on the osmotic pressure difference between the feed and the draw solution with little  
771 or no external hydraulic pressure. FO membranes are dense, similar to the RO membrane but  
772 thinner to limit the internal concentration polarization phenomenon. Hybridization of FO-  
773 AnMBR systems can be classified into various types based on membrane configuration (side-  
774 stream or submerged), draw solution (DS) regeneration system (open without regeneration or  
775 close with regeneration) and objective of treatment (wastewater treatment, resource recovery,  
776 wastewater pre-concentration and/or reclamation).

777 FO can also be coupled with AnMBR as pre or post-treatment for concentration purposes; in  
778 that case, AnMBR features a UF/MF membrane and the FO process is placed upstream or  
779 downstream of AnMBR. Pre-concentration of domestic wastewater may allow for higher  
780 COD load in the AnMBR and therefore optimized operation (Bao et al., 2019; Ferrari et al.,  
781 2019; Wang et al., 2016; Zhang et al., 2014) and post-treatment allows the nutrient recovery  
782 (Ansari et al., 2017; Xue et al., 2016) or concentration of volatile fatty acids (Blandin et al.,  
783 2019).

784 Current limitations of FO were mainly low flux associated with the first generation of  
785 membranes (<10LMH) and salt accumulation due to feed salt accumulation and reverse salt  
786 transport from the draw solution. High cost associated with the draw solution (DS)  
787 regeneration processes (Wang et al., 2016a) such as pressure-driven system (Nanofiltration or  
788 Reverse osmosis) temperature-driven (membrane distillation) or electricity-driven  
789 (electrodialysis) is also a point of concern (Ansari et al., 2017; Wang et al., 2016a). Water  
790 flux in FO processes also reduces due to the phenomenon of concentration polarization (CP).  
791 It makes the feed solution more concentrated on one side of the active layer and dilutes the  
792 draw solution on the other side thus, decreasing the effective osmotic pressure driving force.

793 Based on membrane orientation i.e. active layer facing feed (AL-FS) and active layer facing  
 794 draw side (AL-DS), CP has been classified into four categories i.e. concentrative and dilutive  
 795 external concentration polarization (ECP) (ECP takes place on the membrane surface);  
 796 concentrative and dilutive internal concentration polarization (ICP) (ICP occurs inside the  
 797 support layer) (Shaffer et al., 2015). Figure 4 illustrates these phenomena.



798  
 799 **Figure 4 Illustration of internal and external concentration polarization across an**  
 800 **asymmetric FO membrane module. Adapted from (Gulied et al., 2020).**

801 However, recent development regarding new membranes and modules for osmotic membrane  
 802 bioreactor allowing higher fluxes and very high rejection rate (including trace organic  
 803 contaminants) as well as improved operation in submerged mode to limit clogging of  
 804 challenging streams has opened doors to a new generation of FO-AnMBR (Blandin et al.,  
 805 2018b; Blandin et al., 2016a). Salinity build-up is an inherent problem of the osmotic system  
 806 and can be limited by the choice of appropriate draw solution. Finally, to limit draw recovery

807 cost, synergies can be found with other processes such as fertigation, use of waste heat (MD)  
808 or combined desalination and water reuse.

#### 809 **3.4.2. Performance comparison of FO-AnMBR with conventional AnMBR**

810 FO-AnMBR demonstrated higher removal efficiencies (96% organics, 100% total phosphate  
811 and about 62% ammonia nitrogen removal) than conventional AnMBR due to the use of  
812 dense FO membranes (Chen et al., 2014). FO-AnMBR has also shown a higher methane  
813 yield i.e. 0.25-0.3 L CH<sub>4</sub>/g COD (treating dilute wastewater under mesophilic conditions)  
814 than conventional AnMBR (0.21 L CH<sub>4</sub>/g COD at 25°C). It was reported that FO-AnMBR  
815 has lower fouling propensity than conventional MBRs due to the absence of hydraulic  
816 pressure and low flux. Due to this, foulants are not pushed on the surface of the membrane  
817 and hence result in less cake fouling.

818 A study by Chen et al. (2014) found out that the COD removal efficiency of FO-AnMBR was  
819 higher than the conventional AnMBR. Average COD removal efficiency was 96.7% (with  
820 cellulose acetate membrane) whereas in conventional AnMBR it varies from 28 to 90%.  
821 Thanks to the dense FO membrane, FO-AnMBR operation also resulted in almost complete  
822 phosphate removal and 60% removal of ammonia nitrogen which is favorable compared to  
823 conventional AnMBR with almost no removal of ammonia nitrogen. In addition to higher  
824 rejection, the longer residence time in the FO-AnMBR can lead to advanced biodegradation  
825 of organics resulting in lower COD in the supernatant and permeate. For example,  
826 supernatant COD was observed to be reduced from 210 mg/L to 180 mg/L as the operating  
827 time proceed (Chen et al., 2014). Similar behavior was observed by (Hou et al., 2017) where  
828 initially COD of the bulk increases and then after reaching a maximum value it dropped  
829 down with an increase in bio-degradation rate and a decrease in flux.

830 Table 5 summarized the COD removal efficiency and methane yield of different FO-AnMBR  
831 and conventional AnMBR units. Methane yield obtained was in the range of 0.21 to 0.3 L  
832 CH<sub>4</sub> / g COD (Gu et al., 2015; Chen et al., 2014); similar to that of AnMBR (i.e. 0.23 to  
833 0.27 L CH<sub>4</sub> / g COD at 20-35 °C in Martinez-Sosa et al., (2011)).

#### 834 **3.4.3. Fouling and salinity build-up in FO-AnMBR**

835 Flux decline (from 9.5 to 3.5 LMH) in continuous FO-AnMBR operation was observed  
836 (Chen et al., 2014) with an increase in conductivity from 1 to 20.5 mS/cm. This decline was  
837 attributed to two phenomena: membrane fouling and salinity increase. Membrane fouling  
838 occurred because of the attachment of foulants to the membrane surface thereby increasing  
839 hydraulic resistance and reducing flux whereas retention of feed solutes (by the dense FO  
840 membrane) and the reverse salt flux coming from the draw solution are the cause of the latter.  
841 A similar study on osmotic MBR highlighted that salinity increase may alter bacteria  
842 efficiency leading to deflocculating and enhanced fouling (Blandin et al., 2018a). Moreover,  
843 the cake layer disposition on the membrane hinders the back diffusion of the salt, resulting in  
844 the buildup of salt near the membrane surface. It could result in cake-enhanced osmotic  
845 pressure and lead to a reduction in the net driving force.

846 The presence of both protein and polysaccharides contributes to biofouling; A review (Wang  
847 et al., 2016a) commented that both EPS and SMP played a significant role. However, it was  
848 observed that the biofouling in the FO membrane was mostly external and nearly fully  
849 recoverable. When the membrane was rinsed and the supernatant was discharged to bring  
850 conductivity back to 1 mS/cm after one cycle of operation (22 days), 94% of initial flux was  
851 recovered (Chen et al., 2014).

#### 852 **3.4.4. Fouling mitigation in FO-AnMBR**

853 Fouling mitigation can be done by modification of membrane properties, improved  
854 configuration and optimization of driving force, operating parameters and control strategies  
855 (Wang et al., 2016a). The effect of the driving force by using different concentrations of draw  
856 solution (NaCl) on membrane flux and fouling was studied (Wang et al., 2019). With  
857 increased driving force, initial flux increases but beyond a certain critical concentration the  
858 flux decline rate increased drastically (0.57 LMH/day to 1.24 LMH/day when concentration  
859 increased from 1 to 2 M). Moreover, an increase in the driving force resulted in more drastic  
860 fouling (fouling layer thickness increases from 35.20  $\mu\text{m}$  to 96.5  $\mu\text{m}$  with a concentration  
861 increase of 0.25 M to 2 M). Major constituents of the bio-foulant layer were found to be  $\beta$ -D-  
862 glucopyranose, proteins and bacterial cells whose bio-volume also increased with DS  
863 concentration. It is assumed that higher initial water flux increased the foulant deposition rate  
864 and raised the membrane resistance thereby, aggravated the fouling resulting in severe flux  
865 drop. As for MBR, evidence of critical flux has been mentioned for the osmotic system when  
866 using a new generation of FO membrane (Blandin et al., 2018a; Blandin et al., 2016b) and  
867 therefore similar behavior probably occurs in FO-ANMBR. Operating below critical flux  
868 would lead to a more stable and sustainable operation.

869 (Hu et al., 2017) compared the impact of NaCl and  $\text{MgCl}_2$  as draw solutions on fouling  
870 behavior. Flux decline rate using  $\text{MgCl}_2$  (0.3 LMH/day) was more severe compared to NaCl  
871 (0.16 LMH/day) indicating more fouling propensity. Moreover, the fouling layer in the case  
872 of  $\text{MgCl}_2$  was 10  $\mu\text{m}$  thicker than that of using NaCl as a draw solution. Enhanced fouling  
873 with  $\text{MgCl}_2$  draw solution could be caused by the bridging ability between magnesium  
874 (divalent) ion and EPS. It was also observed that the fouling potential of the TFC polyamide  
875 membrane was higher compared to CTA membranes since fouling was reversible up to 94%

876 of flux recovery for the CTA membrane (section 4.3) but it was not the case with TFC  
877 probably as a result of higher flux.

#### 878 **3.4.5. Salinity build-up mitigation in FO-AnMBR**

879 The salinity build-up in wastewater is a major issue to be resolved in FO-AnMBR which is  
880 inherent to the FO process (dense membrane). The accumulation of salts reduces permeate  
881 flux due to the loss of the osmotic gradient and can negatively impact the biological process.

882 One way to mitigate salt accumulation is to control reverse salt flux from the draw solution to  
883 the FO membrane. Using divalent salts (more rejected by the FO membrane) this could be  
884 envisioned. The study showed that (Tang et al., 2014) salt accumulation was greater when  
885 sodium chloride was used as the draw solution (salinity = 35 mS/cm) than other salts such as  
886 sodium sulfate (salinity = 11 mS/cm) under the same operating conditions in FO-AnMBR.  
887 Chloride ion has a smaller size than sulfate, thus exhibiting higher reverse salt flux.  
888 Moreover, the negatively charged FO membrane would offer more repulsion to sulfate ion  
889 than chloride due to differences in their charge and size. However, use of sodium sulfate  
890 lowered the methane production (only 2.6 % of methane in biogas compared to 12.9% as  
891 NaCl was used as the draw solution). The presence of sulfate can increase the production of  
892 sulfur-reducing bacteria hindering methanogens thereby affecting methane production. Thus,  
893 the use of a sulfate-based draw solution needs to be considered very carefully if significant  
894 methane generation is required.

895 Playing with HRT and SRT (wasting sludge to discharge accumulated salts) is an alternative  
896 to mitigate salt accumulation but should be controlled carefully and reduced SRT may  
897 negatively impact the process economics (Wang et al., 2016a). Another way to mitigate  
898 salinity build-up during the operating cycle is to couple a MF membrane with FO-AnMBR  
899 (Wang et al., 2017). Cellulose triacetate (CTA) FO membrane and polyvinylidene fluoride

900 (PVDF) MF membrane was submerged together into an anaerobic bioreactor with 0.5M NaCl  
901 of draw solution at 25 °C and at a biogas sparging rate of 2 L/min. It was found that the  
902 conductivity of the mixed liquor remained stable and low (2.5 – 4 mS/cm) compared to the  
903 previous study using FO-AnMBR, the MF membrane acting as the salt leak. The  
904 performances of both MF and FO membranes were in accordance with conventional AnMBR  
905 and FO-AnMBR yielding 90% and 96% of TOC removal efficiency, respectively. Also,  
906 methane yield ranged from 0.25 to 0.28 L CH<sub>4</sub>/g COD. However, the fouling on the FO  
907 membrane was severe compared to conventional FO membranes because the permeate flux  
908 could not be recovered after chemical cleaning at the end of the operation (101 days).

### 909 **3.5. Microbial Electrolysis Cell-AnMBR**

910 Applying an electrical field in an anaerobic membrane process by coupling MEC and  
911 AnMBR is an innovative way to reduce membrane fouling (Ding et al., 2018). A PVDF  
912 hollow fiber membrane was submerged into a MEC where titanium mesh and carbon were  
913 used as cathode and anode respectively at external circuit resistance of 10 Ω and 36 hours of  
914 HRT. Voltage ranging from 0 to 1.2 V was employed to study the removal efficiency and  
915 fouling behavior of the system. COD removal with the voltage applied was about 20 %  
916 higher than that without it. However, COD removal was not constant with time but decreased  
917 slowly indicating a negative effect of high voltage on degradation kinetics of organic  
918 contaminants. This degradation was due to increased plasmatorrhesis, reduced metabolic  
919 activity and decrease in microorganism growth rate. Hence, there is a need to optimize  
920 voltage for maximum efficiency.

921 The rapid decline in resistance from 400 Ω at 0.4 V to 66.67 Ω at 0.6 V indicates the potential  
922 for fouling mitigation. Moreover, with an increase in applied voltage, cycle length, defined as  
923 the time of operation before cleaning is required, increased (60 h at 0.4 V to 98 h at 1 V)  
924 showing a reduction in cleaning requirement and hence lower fouling. The increasing voltage

925 from 0 V to 1 V in MEC-AnMBR increases the zeta-potential from 22.3 mV to 30.9 mV.  
926 Higher zeta-potential of sludge resulted in less agglomeration of it on the membrane surface  
927 due to higher negative charge of the sludge, thereby reducing cake formation due to  
928 increasing electrostatic repulsion between the sludge and the membrane. In addition, as per  
929 DLVO theory, electrostatic repulsion will reduce the stability and compactness of the cake  
930 layer thus improving permeability. A decrease in protein to carbohydrate ratio of EPS (P/C  
931 ratio) was observed due to a reduction in proteins in EPS (positively charged amino acid  
932 groups) resulted from lower microorganism activity and reduced substrate adsorption on  
933 sludge. This will also result in increase of negative charge of the cake as less amino groups  
934 will be present to neutralize the negative charge of carboxyl groups and phosphoric acid  
935 groups and thus making it less sticky. A decrease in sludge viscosity from 4.3 mPa.s to 3.3  
936 mPa.s at 0 V and 1V, respectively was observed and the sludge adsorption capacity was  
937 reduced. All these factors result in a decreasing fouling rate.

938 Another paper (Zhang et al., 2017) studied a system made by the integration of Microbial  
939 Electrolysis Cell with FO-AnMBR. The MEC coupled FO-AnMBR configuration aimed at  
940 providing better methane production, reducing fouling and mitigating concentration  
941 polarization. The author used stainless steel mesh as a cathode which was placed in contact  
942 with the membrane (cellulose triacetate based) and carbon brush as the anode. The applied  
943 voltage was 0.5V. It was found that the conductivity increase rate in FO-AnMBR was 0.11  
944 mS/(cm.d) whereas in MEC coupled FO-AnMBR was 0.08 mS/(cm.d) indicating less reverse  
945 solute flux. The reverse flux of acetate ion was lower due to the repulsion created by the  
946 negatively charged cathode and to maintain charge balance of draw solution reverse flux of  
947 magnesium ion was also reduced. Application of electricity and the resultant reduction in  
948 reverse solute flux decrease the salinity accumulation which indeed offers benefits like better  
949 methane yield (11.07% more methane in biogas with MEC coupling compared to



950 conventional FO-AnMBR), less fouling (8.93% less loosely bound EPS and 19.3% less SMP  
951 in case of MEC coupled FO-AnMBR compared to FO-AnMBR) and improved membrane  
952 flux (operating cycle prolonged to 1.3 times the simple FO-AnMBR). In addition to this MEC  
953 coupled FO-AnMBR provided 9.48% more COD removal efficiency compared to FO-  
954 AnMBR operated under the same conditions due to degradation of the organic compound at  
955 carbon anode in addition to anaerobic degradation. Moreover, the author explained that  
956 protein-based EPS and SMP in the fouling layer which cause severe membrane fouling was  
957 less in MEC coupled FO-AnMBR. This could be the result of electrostatic repulsion offered  
958 by cathode to negatively charged protein.

959 A similar configuration but with additional recovery mode was introduced by one study (Hou  
960 et al., 2017). FO-AnMBR coupled with the electrolysis system was operated at two modes,  
961 initially as MEC where two carbon anodes sandwiching a Ni-based cathode were used for  
962 biogas production. Later it was converted into recovery mode by placing the ionic exchange  
963 membrane between the electrodes. This configuration provided benefits that include diffusion  
964 of  $\text{NH}_4^+$  through the FO membrane thus improving effluent quality, phosphorous ion  
965 recovery resulting in reduced scaling, recovery of sulfate ion thus reducing methane loss due  
966 to sulfate reduction and desalination of the bulk solution resulting in less osmotic pressure  
967 loss.

968 During the operation, salinity build-up in MRC mode was less (0.7 to 12.7 mS/cm in 11 days)  
969 compared to MEC mode (0.7 to 17 mS/cm in 11 days). This was due to ion exchange during  
970 the recovery mode which was confirmed by an increase in conductivity of recovery solution  
971 from 1,1 to 11.25 mS/cm. Corresponding to salinity build up the flux decline was observed  
972 from 8.7 to 4 LMH. However, SEM analysis of the membrane indicated that bio-fouling  
973 accounts for this decline rather than scaling, therefore, indicating the positive impact of MEC  
974 coupling on scaling control in FO-AnMBR. The author also observed 97.3% flux recovery

975 after membrane rinsing indicating that the reversible fouling dominates. During MRC mode,  
976 41 to 65% of PO<sub>4</sub>-P was recovered from the bulk at the end of each stage, which not only  
977 helped in alleviating scaling but also added to nutrient recovery.

978 Moreover, the system provided the current generation with average coulombic efficiency of  
979 40% and provided a methane yield of 0.15 L CH<sub>4</sub>/g COD. It can be concluded that the  
980 coupling of MEC and FO-AnMBR systems could be advantageous not only in terms of  
981 scaling mitigation and salinity control but also for nutrient and energy recovery. However,  
982 further research is needed to develop technology at a larger scale and wider operating  
983 conditions.

### 984 **3.6. Anaerobic Electrochemical Membrane Bioreactor (AnEMBR)**

985 AnEMBR combines MEC with AnMBR in such a way that the membrane act as one of the  
986 electrodes and serves in both hydrogen generation and effluent filtration. This novel  
987 technique uses a polymer-based hollow fiber membrane hybridized with an electrically  
988 conductive material such as Ni (Sapireddy et al., 2019; Katuri et al., 2014), graphene (Werner  
989 et al., 2016) or carbon nanotubes (Yang et al., 2019; Yang et al., 2018). The application of the  
990 voltage in this configuration has not only improved hydrogen generation but also improves  
991 the fouling reduction.

992 Yang et al., (2018) studied the fouling mechanism and performance of AnEMBR that uses  
993 carbon nanotubes HF membrane as cathode and titanium mesh as the anode. It was found that  
994 the application of voltage has a benefit in membrane fouling reduction. TMP in the case of  
995 AnEMBR (applied voltage = -1.2V) was 35 kPa after 30 days of operation which was much  
996 lower compared to the control reactor which observed 60 kPa (conventional AnMBR). It was  
997 explained that these results were observed because negative potential repelled the negatively  
998 charged pollutants from the membrane reducing the potential of cake formation. The electric

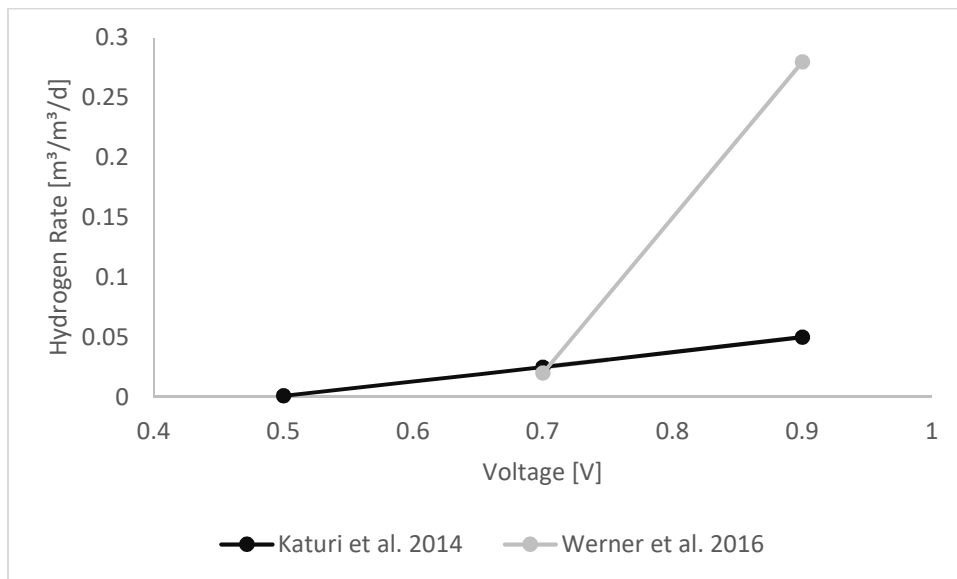
999 field also aided in the destruction of high molecular weight molecules and prevented the  
1000 formation of complex cross-links between organics and divalent metal ions thus alleviating  
1001 fouling. Moreover, AnEMBR showed better performance, 98% COD removal compared to  
1002 95% COD removal in conventional AnMBR. Better performance can be explained by the fact  
1003 that the creation of electro-active biofilm on cathode surface improved organic degradation  
1004 and electric potential might have accelerated the biological degradation of organic matter.

1005 Another study (Yang et al., 2019), compared the AnEMBR with and without voltage  
1006 application with a conventional AnMBR and found out similar results for COD removal and  
1007 TMP i.e. in the case of electro-assisted AnEMBR (-1.2 V applied voltage). COD removal was  
1008 higher than 95% and TMP was 35 kPa, whereas for AnEMBR and AnMBR COD removal  
1009 was less than 95% and TMP was 50 kPa and 60 kPa, respectively. Thus, confirming that the  
1010 application of voltage can not only alleviate fouling but can also improve COD removal  
1011 efficiency based on the above-mentioned reasons.

1012 It further investigated the EPS content of fouled membrane to study fouling and found that  
1013 protein EPS (almost 180 mg/L compared to >180 mg/L) and carbohydrates EPS (almost 150  
1014 mg/L compared to >150 mg/L in AnEMBR without voltage application and >250 mg/L in  
1015 AnMBR) were lower in electro-assisted AnEMBR. Lower EPS prevents the formation of the  
1016 gel layer, that results in severe fouling, since sticky protein EPS is limited in quantity. Lower  
1017 protein EPS means lower positively charged amino acids and thus better repulsion by  
1018 negative electric field thus reduction in sludge deposition. It was also observed that the  
1019 application of voltage increases the CH<sub>4</sub> generation (an increase of more than 40 mL/g VSS d  
1020 in methane yield compared to the other two reactors) by enhancing the growth of  
1021 microorganism over time thus improving volatile fatty acid's degradation and improving  
1022 activity of methane-producing microbes. Higher methane recovery had a very positive effect

1023 on the overall energy balance of the system and maximum surplus energy of 51.46 kJ/day  
1024 was calculated in the system.

1025 Other studies (Werner et al., 2016; Katuri et al., 2014) also found that increase in applied  
1026 voltage in AnEMBR systems help in fouling mitigation. At higher voltage, hydrogen  
1027 production is increased (Figure 5) which acts as an additional scouring agent and reduces  
1028 fouling. Increased hydrogen production was because at higher voltage COD removal was  
1029 faster (due to high current density) hence cycle time was slower, so hydrogen was dominant  
1030 because the rate of hydrogen production was higher than the rate of hydrogen consumption  
1031 by methanogenic bacteria.



1032  
1033 **Figure 5 Hydrogen Rate at Different Applied Voltage**

1034 One research (Sapireddy et al., 2019) demonstrates the effect of cathode surface area by  
1035 operating three AnEMBR, for acetate removal, with Ni-based Hollow fiber membrane as the  
1036 cathode, of specific cathode surface area (SCSA) of 2 m<sup>2</sup>/m<sup>3</sup>, 4 m<sup>2</sup>/m<sup>3</sup> and 8 m<sup>2</sup>/m<sup>3</sup>  
1037 respectively. The applied voltage was 0.7 V and the flux of 16 LMH. Acetate removal was  
1038 almost similar in the case of all three AnEMBR and range between 77 to 94% throughout the  
1039 operation. Coulombic efficiency is higher at the beginning of operation when hydrogen is  
1040 dominant (cathode recovery of hydrogen being 77.00 ± 3.05% and that of methane being 0.82

1041  $\pm 0.26\%$  in case of SCSA of  $8 \text{ m}^2/\text{m}^3$ ) in gas and decreases as methane generation (cathode  
1042 recovery of hydrogen being  $24 \pm 2.29 \%$  and that of methane being  $16.9 \pm 1.02 \%$  in case of  
1043 SCSA of  $8 \text{ m}^2/\text{m}^3$ ) increases. Hydrogen being an electron donor account for this behavior.

1044 In terms of biofouling AnEMBR with SCSA of  $8 \text{ m}^2/\text{m}^3$  showed the best performance with  
1045 the lowest TMP. Higher hydrogen flux and increased bubble frequency with more proportion  
1046 of small-sized bubbles are responsible for this behavior. Hydrogen was found to be more  
1047 efficient in TMP control because of its faster evolution (less time required for molecular  
1048 diffusion), higher gas velocity due to less density ( $0.0898 \text{ kg}/\text{m}^3$ ) than methane ( $0.656 \text{ kg}/\text{m}^3$ )  
1049 and higher gas volume since one mole of acetate produces 4 moles of hydrogen and one mole  
1050 of methane. Higher bubble frequency produced more scouring effect due to increased bubble  
1051 velocity and thus result in lower TMP. Moreover, smaller the size of the bubble better will be  
1052 the scouring since small bubbles have higher slip velocity (velocity of gas relative to liquid)  
1053 and shear force.

### 1054 **3.7. Vibration application in AnMBR**

1055 Vibratory shear enhancement process (VSEP) was first commercially introduced in flat sheet  
1056 membranes by Armando and Culkin (New Logic International Inc, 1992). Shear forces  
1057 generated by vibrations result in increased turbulence which enhances the removal of cake as  
1058 the forces that were induced by shear dominated the physicochemical interactions between  
1059 membrane and foulants and reduce cake layer fouling. Vibrations have a limited effect on the  
1060 small foulants that can adsorb inside pores and cause pore blocking. However, since in  
1061 AnMBR treating domestic wastewater cake layer fouling phenomenon dominates (Chen et  
1062 al., 2014) use of vibrations can serve as a good mitigation technique. There are two ways of  
1063 vibrations generation; shaker generated oscillations in liquid with a stationary membrane  
1064 and/or vibrating or rotating membrane on a rotor where the liquid is held stationary (Kola et  
1065 al., 2014). One study (Kola et al., 2014) demonstrated the effectiveness of transverse

1066 membrane rotation in HF AnMBR with increasing MLSS. At increased MLSS, conventional  
1067 methods of fouling mitigation like gas sparging are not effective because increased viscosity  
1068 greatly reduces the velocity of rising bubbles and thus reduces shear force. It was observed  
1069 that at a vibration frequency of 6.7 Hz, a constant flux of 26 LMH could be maintained with  
1070 increase in MLSS from 0.005 g/L to 5 g/L which in the absence of vibration decreased from  
1071 16 LMH (0.005 g/L MLSS) to 1 LMH (5 g/L MLSS). This indicated that vibrations produce  
1072 enough shear to counter fouling even with increasing MLSS. Increasing the frequency of  
1073 vibrations from 6.7 Hz to 20 Hz effects the hydrodynamic conditions in the reactor resulting  
1074 in more turbulence (Reynolds number increases from 365 to 1095 respectively) which creates  
1075 the higher shear rate i.e. (1372 1/s to 30108 1/s) thus reducing fouling and increasing critical  
1076 flux from 26 LMH to 51 LMH. Additionally, when compared with gas sparging and cross  
1077 velocity methods at a constant flux of 30 LMH and MLSS of 26 mg/L, the time required to  
1078 reach a TMP of 60 kPa in case of vibration application was 3 times more. Similar results  
1079 were observed by others (Ruigómez et al., 2016a) when they compared rotation with gas  
1080 sparging and found that in case of rotation better cake redispersion was observed which  
1081 improved the cycle duration by 10 times compared to gas sparging.

1082 Ruigómez et al., (2016a) observed that the fouling resistance drop from 0.16 kPa/s to 0.01  
1083 kPa/s with the membrane rotation of 0 to 260 rpm respectively at a permeate flux of 14 LMH.  
1084 They also found that the turbulence promotor efficiency (defined as the decrease in fouling  
1085 resistance compared to initial value with mitigation technique employed) for membrane  
1086 rotating at 260 rpm was 96% compared to gas sparging with specific gas demand of 3.8  
1087  $\text{m}^3/(\text{m}^2.\text{h})$  which has the turbulence promotor efficiency of 44.4% at a flux of 8 LMH in both  
1088 the cases. This difference was observed because, in the case of membrane rotation, vibration  
1089 energy is more uniformly distributed compared to sparging thus provides better shear.  
1090 Another study (Mertens et al., 2019) also found improved fouling mitigation (10 times less

1091 fouling resistance at a flux of 20 LMH) by increasing shear rate from  $0 \text{ s}^{-1}$  to  $605 \text{ s}^{-1}$  in their  
1092 magnetically induced membrane vibration (MMV) AnMBR system. They also found out that  
1093 with a system of 4 modules, the contribution of the MMV system to total energy was only  
1094 12% of total consumption.

1095 It was found that by increasing vibration from 0 Hz to 13.3 Hz, the critical flux increase by  
1096 3.7 times. However, the critical flux at 20 Hz was only 24 % of that at 13.3 Hz (Kola et al.,  
1097 2014), which is because, at higher vibrations, shear forces dominate Brownian, inertial and  
1098 drag forces and thus causing particles to be moved away from the membrane surface. As a  
1099 result of this, pore-blocking (on which vibrations have an insignificant effect) dominates. It is  
1100 therefore important to select the vibration frequency carefully to optimize fouling mitigation  
1101 with minimum energy demand.

1102 The effectiveness of vibrations application can be further enhanced by coupling it with  
1103 periodic relaxation or backwash. For the operation of 25 days (flux = 30 LMH) when  
1104 AnMBR was run at a vibrational frequency of 4.2 Hz coupled with relaxation or backwashing  
1105 after 30 minutes intervals for 30 seconds (Kola et al., 2014) time required to reach a TMP of  
1106 60 kPa was 100 hours for relaxation and 60 hours for backwash compared to 28 hours  
1107 (approximately) with vibrations alone. The authors explained that in case of relaxation, the  
1108 cake layer is loosened during relaxation and then application of vibration removed foulant  
1109 thus improving the performance. In the case of backwash, when the cake layer was removed  
1110 or restructured during washing (which is not the case in relaxation thus the cake layer serves  
1111 as a protective layer) pore-blocking phenomenon may increase since the effectiveness of  
1112 backwashing was not as high as periodic relaxation. On contrary to this, one study (Mertens  
1113 et al., 2019) found backwashing to be 10 times more effective than relaxation in a 10-day  
1114 operating cycle at a flux of 20 LMH and shear rate of  $484 \text{ 1/s}$  where backwashing (or  
1115 relaxation) time was 2 minutes after 8.5 minutes. Similarly, another study (Ruigómez et al.,

1116 2016a) conducted experiments with a filtration cycle of 12.7 minutes and cleaning duration of  
1117 30 s at a rotational speed of 180 rpm and permeate flux of 8 LMH and found out that the  
1118 internal fouling resistance is less in case of backwashing (11% of total) compared to that in  
1119 case of relaxation (15% of total). They also coupled both backwashing and relaxation and  
1120 found that optimal results were obtained at a backwashing ratio of 0.67 and a specific TMP of  
1121 20 kPa. Under these conditions, reversible fouling resistance was 41% of the total and  
1122 internal fouling resistance was 12% of the total. Moreover, the highest value of dispersed  
1123 TSS (solid concentration of  $\sim 6.5$  gSS/m<sup>2</sup>) was obtained under these operating conditions.

1124 From this discussion, we can conclude that the application of vibration in AnMBR operation  
1125 can serve as an effective fouling mitigation technique especially if coupled with other  
1126 cleaning techniques like backwashing and relaxation. However, the best-coupled combination  
1127 is case dependent as it is controlled by many factors like cycle duration, permeate and  
1128 backwashing flux, days of operation and cleaning time. Therefore, it is important to develop  
1129 the best combination, experimentally, for employed operating conditions. To further  
1130 understand the sustainability of this technique, studies must be conducted developing  
1131 understanding of fouling behaviors and establishing energy balances to see the energy  
1132 efficiency of vibrational systems compared to other available mitigation techniques.

### 1133 **3.8. Biological mitigation – Quorum Quenching**

1134 Quorum sensing (QS) is a communication technique between the cells that is used by  
1135 microorganisms to control their activities like attachment or detachment, EPS formation,  
1136 biofilm production and mobility. The communication is done by a generation of signal  
1137 molecules that are called autoinducers e.g. N-acryl homoserine lactone (AHL). Deactivation  
1138 of autoinducers results in disruption of communication between the cells and this process of  
1139 deactivation is called Quorum Quenching (QQ). QQ is carried out by using QQ enzymes or



1140 bacteria that can be isolated from the sludge (Aslam et al., 2018). QQ enzymes are divided  
1141 into two main types i.e. AHL-Lactonase and AHL-acylase. The former having a cleavage  
1142 effect on the ester bond of the lactose ring and later breaks the amide bond. Additionally, the  
1143 oxidoreductase enzyme also functions as a QQ enzyme by converting AHL to 3-hydroxy  
1144 AHL thus preventing signal molecule to form bio-film (Kim et al., 2014a). Enzymatic QQ  
1145 has some disadvantages such as high cost, instability and need for purification. These are  
1146 overcome by the employment of QQ bacterial (Id et al., 2018). Kim et al. (2014a) have  
1147 isolated 225 AHL degradation bacteria from waste sludge.

1148 Many studies in the literature have proved the effectiveness of quorum quenching in aerobic  
1149 MBRs where quorum quenching bacteria has been employed in carriers like QQ beads, QQ  
1150 cylinders, QQ sheets or QQ vessels (Ergön-can et al., 2017; Hyun et al., 2017; Won et al.,  
1151 2016; Weerasekara et al., 2014). However, not much work has been done so far in AnMBR.  
1152 The first study that has tested the effectiveness of QQ in AnMBR was published in 2019 (Liu  
1153 et al., 2019) followed by two more in 2020 (Xu et al., 2020a, 2020b).

1154 Liu et al., (2019) tested four different bacterial species by entrapping them in alginate beads  
1155 and then selected microbacterium sp. (a facultative anaerobe) for long term study due to its  
1156 higher COD removal efficiency and higher methane production. QQ bacteria were tested in a  
1157 sequence of three phases i.e. without beads (control), with empty beads (EB) and with QQ  
1158 beads (QQB) respectively (Liu et al., 2019). Membrane flux was kept at 8 LMH and HRT of  
1159 18.5 hours was used. COD removal efficiency during all three phases was close (97.98% for  
1160 control, 98.84% for EB and 98.75% for QQB) however there is a significant difference in  
1161 time required to reach TMP jump which is an indicative phenomenon for biofilm and bio-  
1162 cake structural change and represent fouling in AnMBR (Zhang et al., 2006). The time  
1163 requires to reach TMP in three phases was 6, 8 and 45 days, respectively. Slight improvement  
1164 in the EB phase was probably due to the scouring effect. These results show the potential of

1165 QQB in fouling control in AnMBR. The effectiveness of the QQB phase was described by  
1166 the analysis of AHLs concentration in the fouling layer that showed that the AHLs  
1167 concentration (responsible for biofouling) in the QQB phase was lower than the control  
1168 phase.

1169 Authors (Liu et al., 2019) also observed a significant reduction in EPS content (responsible  
1170 for cake fouling (Chen et al., 2017a)) i.e. 15.18% reduction in the bulk phase and 75.32%  
1171 reduction in the foulant layer. The protein content of EPS which is responsible for sticky cake  
1172 fouling (Chen et al., 2017a) was also reduced significantly compared to the control phase  
1173 (80.98% reduction). Although the QQ was found very effective in combating fouling, the  
1174 AHLs degradation by single QQ species declined after 45 days. Xu et al., (2020a) proposed  
1175 the use of a facultative QQ consortium and study its effect of AHLs degradation and fouling  
1176 control in AnMBR.

1177 Xu et al., (2020a) studied three facultative QQ consortium consisting majorly of  
1178 Proteobacteria (abundance > 90%), Firmicutes and Actinobacteria phylum isolated from  
1179 activated sludge obtained from a local domestic wastewater treatment plant. All three  
1180 consortiums were successful in diminishing multiple AHLs (acyl chains ranging from C4 to  
1181 C10) that are responsible for biofouling in AnMBRs. The degradation rate in all cases was  
1182 above 80% whereas without QQ the maximum rate obtained was 28%. It was also found that  
1183 bead entrapped FQQ shows higher alleviation in EPS production (both carbohydrates and  
1184 proteins) compared to free one signifying the importance of immobilization in protection  
1185 against the harsh environment. FQQ with 6 carbon chain (FQQ-C6) showed the best  
1186 performance in protein (72.3%) and carbohydrate (66.53%) reduction indicating its potential  
1187 to mitigate fouling in AnMBR. When bead entrapped FQQ-C6 was used in AnMBR with a  
1188 ceramic membrane of pore size 0.1  $\mu\text{m}$ , 30  $^{\circ}\text{C}$  temperature, gas sparging rate of 0.7 L/min, 30  
1189 days SRT and 17 hours HRT, a total COD removal 91% and methane yield was 0.07

1190 L/gCOD<sub>removed</sub> indicating no negative effect of performance efficiency. Moreover, the  
1191 presence of QQ increased the average operating period of filtration cycles by 75%, indicating  
1192 efficient fouling mitigation. All these results showed the potential of QQ for fouling  
1193 mitigation in AnMBR however, extensive research is required to understand the potential of  
1194 long-term operation and scale-up.

#### 1195 **4. Future perspective**

1196 AnMBR technologies allow positive energy treatment of domestic wastewater with  
1197 optimization of operating conditions and fouling control strategies. Innovative mitigation  
1198 techniques and process configurations are trying to resolve the issues of fouling that account  
1199 for the biggest chunk of energy demand. Recent literature on lab-scale studies has suggested  
1200 interesting and innovative solutions. Hybrid Processes like G-AnMBR, cell entrapment  
1201 AnMBR, FO-AnMBR, MEC-AnMBR and dynamic AnMBR have shown lower fouling rate  
1202 and higher % COD removal and methane yield than conventional AnMBR (Table 6).  
1203 However, these are in very early stages of development and for scale-up, there is a need for  
1204 an extensive study of the fouling mechanism, change in this mechanism with changing  
1205 conditions and energy balance of the whole process. It is also important that the results of  
1206 lab-scale studies should be validated at a larger scale and long-term studies should be  
1207 conducted to broaden the range of practical application.

1208 The application of QQ in AnMBR showed promising results (Table 6) both in terms of  
1209 energy-saving and production, fouling control and % COD removal. It will be interesting to  
1210 apply QQ in other configurations e.g. FO-AnMBR, AnDMBR, etc. and evaluate technical  
1211 and economic feasibility. Another fascinating approach can be the coupling of the electric  
1212 field or vibrations with QQ and study their effect on fouling control. However, these  
1213 technologies require extensive research to understand and express their full potential.

1214 Moreover, methane loss, especially under psychrophilic conditions (10-30 °C) is an area that  
1215 requires more research covering the maximization of methane yield and minimization of  
1216 methane leaks in the dissolved phase. Integration of methane recovery units (like membrane  
1217 degassing units), improvement of membrane materials and reduction of operating energy  
1218 should be given attention.

1219 Other research areas that can positively contribute towards fouling mitigation in AnMBRs  
1220 and help in making them water resource recovery facilities may include,

- 1221 • Improved membrane materials with anti-fouling capabilities;
- 1222 • Coupling of different hybrid processes to combine benefits and minimize  
1223 disadvantages, for example, the coupling of MEC with FO-AnMBR or Vibrations  
1224 with different hybrid AnMBRs;
- 1225 • Post-treatment step integration with AnMBR for nutrient recovery. They may include  
1226 adsorption, ion exchange, microalgae cultivation and forward or reverse osmosis  
1227 (Shahid et al., 2020);
- 1228 • Studies involving complete plant energy assessment to analyze the effect of the  
1229 secondary operation, e.g. DS re-concentration in FO-AnMBR, methane recovery unit  
1230 for psychrophilic AnMBRs, entrapment procedures and sludge modification  
1231 techniques, on net power generation of the process;
- 1232 • Life cycle assessment and cost-benefit studies of processes to evaluate the  
1233 environmental impacts and implications.

## 1234 **5. Conclusion**

1235 Membrane fouling is a serious concern in the treatment of domestic wastewater treatment  
1236 using AnMBR. At the pilot-scale, biogas sparging is the most employed fouling mitigation  
1237 strategy but it accounts for a huge percentage of total energy requirements. Membrane

1238 material, membrane module, reactor configuration, operating conditions and sludge  
1239 properties contribute towards fouling behavior as well as net energy requirements of  
1240 AnMBR. Some hybrid processes have been proved to successfully reduce fouling problems  
1241 of AnMBR and could be more suitable for efficient resource recovery. Additionally,  
1242 applications of the electric field, voltage and vibrations have contributed positively towards  
1243 fouling control. However, increasing their value beyond a certain limit will not only reduce  
1244 performance efficiency but also unnecessarily increase the energy demand. Biological  
1245 techniques like quorum quenching are found to be equally successful in fouling mitigation in  
1246 AnMBR as they were in AeMBR and have good potential in this area especially if coupled  
1247 with other techniques of fouling mitigation.

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**Table 1. Description of different AnMBR configurations for domestic wastewater treatment (GSP: Gas Sparging, PSP: Particle Sparging, FS: Flat sheet, HF: Hollow Fiber, UF: Ultrafiltration, MF: Microfiltration, RMEM: Rotating membrane).**

N°	Configuration	Membrane Module	Material	COD (% removal)	Organic Loading Rate (kg <sub>COD</sub> /(m <sup>3</sup> .d))	References
Config-1	GSP-FS	<b>FS UF</b>	(PES)	>90	0.6 - 1.1	(Martinez-Sosa et al., 2011)
Config-2	GSP-HF1	HF UF	PURON (Polyester)	87	1.03-2.11	(Giménez et al., 2014)
Config-3	GSP-HF2	HF UF	PURON (Polyester)	85	0.3-1.1	(Robles et al., 2012)
Config-4	GSP-HF3	HF UF	PURON (Polyester)	85	1.25-2.24	(Robles et al., 2013)
Config-5	GSP-HF4	HF UF	PURON (Polyester)	>90	0.25-1.24	(Giménez et al., 2014)
Config-6	GSP-HF5	<b>HF MF</b>	RF-1 (PVDF)	87	3	(Mei et al., 2017)
Config-7	GSP-HF6	HF UF	ZeeWeed 500 (PVDF)	93.7	1.1	(Dong et al., 2015; Shin & Bae, 2018)
Config-8	GSP-HF7	HF UF	ZW-10 Zenon (PVDF)	87	1.2 - 1.44	(Gouveia et al., 2015a)
Config-9	GSP-HF8	HF UF	ZW-10 Zenon (PVDF)	90	1.6 - 2.0	(Gouveia et al., 2015b)
Config-10	GSP-HF9	HF UF	GE ZeeWeed 500 (PVDF)	93	3.77-4.97	(Dong et al., 2016)
Config-11	GSP-HF10	HF UF	W-10 Zenon, GE (PVDF)	>90	0.8-1.8	(Evans et al., 2019)
Config-12	<b>PSP-HF</b>	HF UF	Cheil Industries (PVDF)	>90	NA	(Shin et al., 2014)
Config-13	<b>RMEM-HF</b>	HF UF	GE Water & Process Tech (PVDF)	91	NA	(Ruigómez et al., 2016b)

**Table 2. Description of different parameters in various AnMBR configurations (IG-AnMBR: Internally submerged granular AnMBR; ES-AnMBR: Externally submerged membrane bioreactor)**

<b>Configuration</b>	<b>Membrane Configuration</b>	<b>SS or VSS (g/L)</b>	<b>Days of Operation</b>	<b>GSI<sub>m</sub> (Nm<sup>3</sup>/(m<sup>2</sup>.h))</b>	<b>SRT (d)</b>	<b>HRT (h)</b>	<b>References</b>
Config-1	ES-AnMBR	15-21 gSS/L	100	1.22	NA	19.2	(Martinez-Sosa et al., 2011)
Config-2	ES-AnMBR	22 gSS/L	70	0.23	70	6-20	(Giménez et al., 2011)
Config-3	ES-AnMBR	10-30 gSS/L	730	0.17-0.5	30-70	6-36	(Robles et al., 2012)
Config-4	ES-AnMBR	7-32 gSS/L	365	0.23	70	24.5-5.5	(Robles et al., 2013)
Config-5	ES-AnMBR	10-25 gSS/L	172	0.23	28.6-41.1	12.1-28.4	(Giménez et al., 2014)
Config-6	ES-AnMBR	4.7-20.1 gSS/L	340	0.31	NA	2.2	(Mei et al., 2017)
Config-7	ES-AnMBR	4 gSS/L	90	0.15	70	8.5	(Dong et al., 2015)
Config-8	ES-AnMBR	6 gVSS/L	1095	0.81–1.22	Infinite	7	(Gouveia et al., 2015a)
Config-9	ES-AnMBR	0.9-16.1 gVSS/L	1095	0.16–0.32	NA	12.8-14.2	(Gouveia et al., 2015b)
Config-10	ES-AnMBR	5-15 gSS/L	536	NA	40-70	8.5	(Dong et al., 2016)
Config-11	IG-AnMBR	4.30-7.54 gVSS/L	300	NA	60 ± 27	11 ± 3	(Evans et al., 2019)
Config-12	IG-AnMBR	0.600-1.2 gSS/L	485	NA	6.2-36	4.6-6.8	(Shin et al., 2014)
Config-13	ES-AnMBR	21.3 gSS/L	270	NA	270	33	(Ruigómez et al., 2016b)

**Table 3. Water Treatment energy consumptions of different AnMBR configurations having different COD removal, critical flux and transmembrane pressure**

<b>Configuration</b>	<b>Fouling Control Energy (kWh/m<sup>3</sup>)</b>	<b>Critical Flux (LMH)</b>	<b>Total Energy (kWh/m<sup>3</sup>)</b>	<b>TMP (kPa)</b>	<b>References</b>
Config-1	1.28	7	1.66	17.7	(Martinez-Sosa et al., 2011)
Config-2	0.2	10	0.26	8.0	(Giménez et al., 2014)
Config-3	0.2	12-16	0.26	40.0	(Robles et al., 2012)
Config-4	0.2	10-13.3	0.26	<10.0	(Robles et al., 2013)
Config-5	0.19	7-11	0.24	<10.0	(Giménez et al., 2014)
Config-6	0.5	6	0.64	6.0	(Mei et al., 2017)
Config-7	0.08	17	0.1	8.8	(Dong et al., 2015)
Config-8	0.19-0.5	10-14	0.25-0.65	5.0-55.0	(Gouveia et al., 2015a)
Config-9	0.04-0.1	12-14	0.05-0.13	40.0-55.0	(Gouveia et al., 2015b)
Config-10	0.09	25-27	0.11	1.5-30	(Dong et al., 2016)
Config-11	0.09-0.27	7.6-7.9	0.1-0.3	NA	(Evans et al., 2019)
Config-12	0.1	4.1-7.5	0.13	10.0-27.0	(Shin et al., 2014)
Config-13	0.23	10	0.3	1.0-2.5	(Ruigómez et al., 2016b)



*Table 4. Effect of Pore Size on Flux and % COD Removal in AnDMBR*

<b>Configuration</b>	<b>Module</b>	<b>Support Material</b>	<b>The average Pore size of the support (<math>\mu\text{m}</math>)</b>	<b>Operating Flux (LMH)</b>	<b>% COD Removal</b>	<b>References</b>
Internally Submerged	Flat Sheet	Polypropylene	10	2.2	99.5	(Ersahin et al., 2017)
Side Stream	Tubular	Nylon	61	31.25	-	(Siddiqui et al., 2018)
Internally Submerged	Flat Sheet	Nylon	75	22.5	75-90	(Hu et al., 2018b)
Internally Submerged	Hollow cylinder	Polyamide nylon	150	100	80	(Wang et al., 2018)

**Table 5. Performance of Different FO-AnMBRs and AnMBRs treating domestic wastewater (FS: Flat sheet, HF: Hollow Fiber).**

Configuration	Module	Temperature (°C)	Feed COD (mg/L)	Methane Yield (L CH <sub>4</sub> /g COD)	COD % Removal (%)	References
Internally Submerged FO-AnMBR	FS	25	460	0.21	96.7	(Chen et al., 2014)
Internally Submerged FO-AnMBR	FS	35	460	0.25-0.3	95	(Gu et al., 2015)
Internally Submerged FO-AnMBR + MEC + MRC	FS	25	270±10	0.15	>93	(Hou et al., 2017)
Externally Submerged AnMBR	HF	33	445	0.07*	87	(Giménez et al., 2011)
Externally Submerged AnMBR	FS	35-20	630	0.27	>90	(Martinez-Sosa et al., 2011)

**Table 6. Summary of Different Novel Configurations in AnMBR**

<b>Configuration</b>	<b>% COD Removal</b> (%)	<b>Fouling Rate</b> (Pa/s)	<b>Methane Yield</b> (L CH <sub>4</sub> /g COD)	<b>Membrane Flux</b> (LMH)	<b>Organic Loading Rate</b> (kg <sub>COD</sub> /(m <sup>3</sup> .d))	<b>Reference</b>
<b>AnMBR</b>	90	3.33	0.27	7	0.6 - 1.1	(Martinez-Sosa et al., 2011)
<b>G-AnMBR</b>	92	0.005	0.16	7	-	(Chen et al., 2017b)
<b>Entrapped Cell AnMBR</b>	85	0.057	0.08	10.63	0.57	(Juntawang et al., 2017)
<b>AnEMBR</b>	98	0.014	-	23.22*	0.5	(Yang et al., 2018)
<b>MEC-AnMBR</b>	71	-	-	1.8**	5	(Ding et al., 2018)
<b>FO-AnMBR</b>	96.7	0.21	-	6.5**	0.46	(Chen et al., 2014)
<b>Dynamic AnMBR</b>	90	-	0.21	7	0.37	(Li et al., 2016)
<b>AnMBR with Vibrations</b>	-	0.11	-	26***	-	(Kola et al., 2014)
<b>AnMBR with QQ</b>	98.8	0.024	0.34	8	-	(Liu et al., 2019)

\*Average flux provided per unit TMP

\*\*Average value

\*\*\*Critical flux at a frequency of 6.7 Hz

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